

QU'APPELLE RIVER SSARR MODEL
MODIFICATION STUDY

Prepared for: Prairie Provinces Water Board

HYD-5-66

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SYNOPSIS

The Qu'Appelle River SSARR Natural Flow Model was developed to be used to estimate the natural flow in the Qu'Appelle River at the Saskatchewan/Manitoba Boundary. The model has been used since it was developed in 1975, to simulate natural flow in the Qu'Appelle River, however, over this period numerous problems with the model have arisen.

In 1980, The Prairie Provinces Water Board's Committee on Hydrology formed the Qu'Appelle SSARR Model Sub-Committee to discuss and identify what problems in the model needed to be addressed. As a result of the recommendations made by the sub-committee, the Prairie Provinces Water Board acquired the services of Hydrology Service, Saskatchewan Water Corporation to complete the Qu'Appelle River SSARR Model Modification Study.

This report summarizes the modifications that were made to the Qu'Appelle River SSARR Natural Flow Model. Also summarized in this report are simulation results for the 1975 to 1988 period that were obtained using the modified model. The modified model was found to simulate lake elevations and streamflows under both existing and natural conditions that were much more realistic than those obtained using the original model.

Over the 1975 to 1988 period, the modified model simulated apportionment period natural flow volumes in the Qu'Appelle River at the Saskatchewan/Manitoba Boundary which average 19 percent or 38200 dam³ higher than those simulated by the original model. Although all of the modifications made to the model affect the natural flow estimates, the majority of the increase in the natural flow estimates can be attributed to incorporation of groundwater inflow into Last Mountain Lake and the Fishing Lakes, and compensation for increased evaporation due to artificially high lake levels under existing regulated conditions.

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CHAPTER I
INTRODUCTION

1.1 Background

On October 30, 1969, Canada and the Provinces of Manitoba, Saskatchewan and Alberta entered into an agreement to share the flow and to consider the quality of eastward flowing interprovincial streams. This agreement, the Master Agreement on Apportionment, permits the Province of Saskatchewan to make a net depletion of one-half the natural flow of water arising in each stream flowing into Manitoba from Saskatchewan plus one-half of the water flowing into the province from Alberta. The Prairie Provinces Water Board, established under Schedule C of the Agreement, was given the responsibility to administer the Agreement.

In the early 1970s, the Prairie Provinces Water Board acquired the services of Water Survey of Canada, Environment Canada, to develop a natural flow model of the Saskatchewan portion of the Qu'Appelle River Basin. In December 1975, the Qu'Appelle River Natural Flow Model was completed. The Qu'Appelle River Natural Flow Model used the Streamflow Synthesis and Reservoir Regulation Model (SSARR) developed by the United States Army Corps of Engineers.

1.2 Purpose of Study

Since the model was completed in 1975, the Qu'Appelle River Natural Flow

Model has been used to estimate the flow in the Qu'Appelle River that would have occurred under natural conditions. However, soon after use of the model commenced, numerous problems with the model became evident.

In 1980, the Prairie Provinces Water Board's Committee on Hydrology formed a sub-committee to study what modifications needed to be made to the Qu'Appelle River Natural Flow Model. The Qu'Appelle SSARR Model Sub-Committee studied the model and identified the deficiencies that needed to be addressed.

In April 1989, the Saskatchewan Water Corporation and the Prairie Provinces Water Board entered into a Memorandum of Understanding. With the signing of this agreement, the Saskatchewan Water Corporation agreed to modify the Qu'Appelle River Natural Flow Model. The Memorandum of Understanding is included in Appendix A. This report summarizes the modifications which were made to the Qu'Appelle River SSARR Natural Flow Model.

1.3 Qu'Appelle River Basin

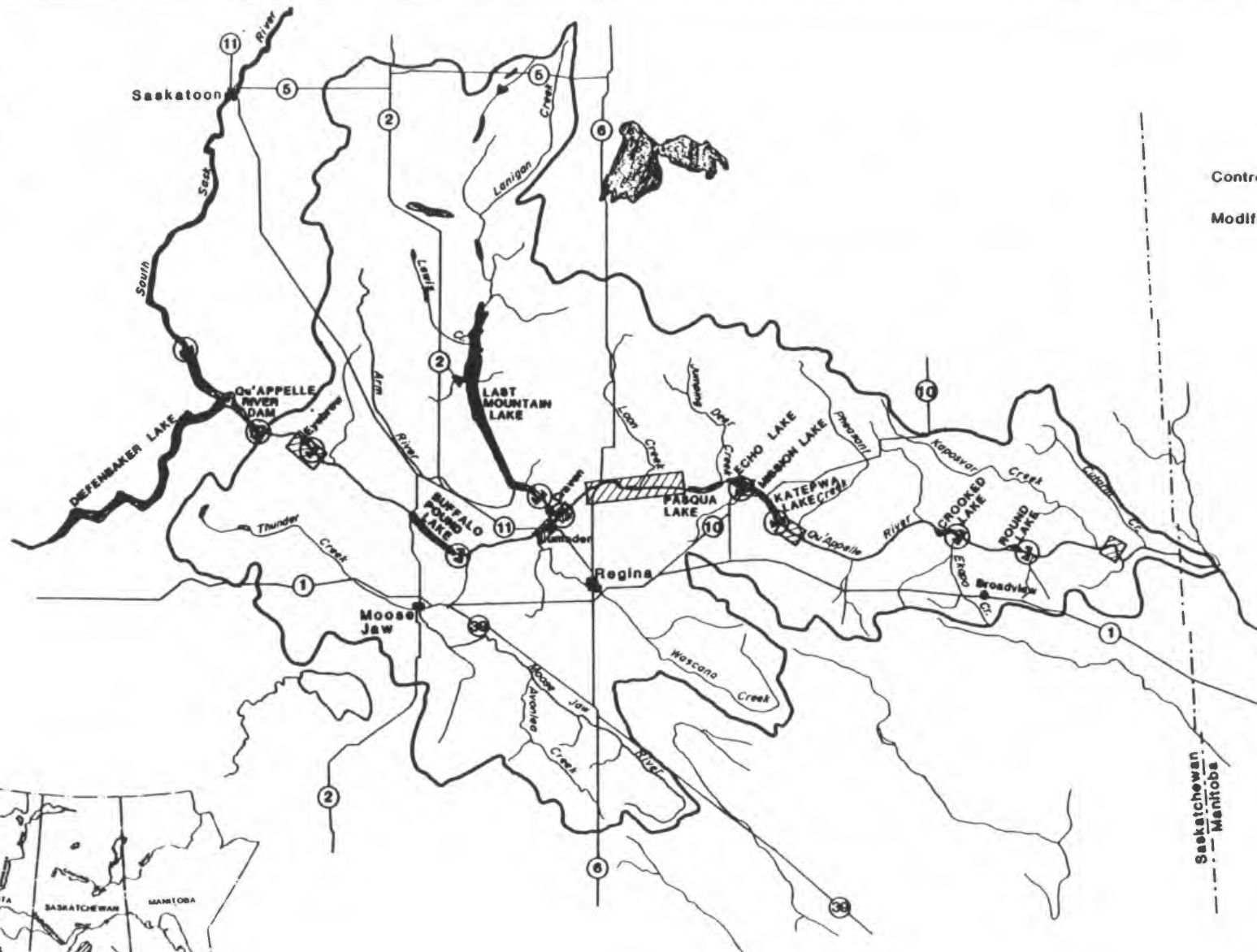
The Qu'Appelle River Basin is located in east central Saskatchewan, as shown in Figure 1.2-1. The basin extends approximately 400 km from its headwaters near the Qu'Appelle Dam on Lake Diefenbaker to its confluence with the Assiniboine River near the Saskatchewan/Manitoba Boundary. The basin encompasses an area of approximately 50000 km².

From below the Qu'Appelle Dam, the Qu'Appelle River flows through Buffalo Pound Lake, continues in an easterly direction and receives waters from two major tributaries, Moose Jaw River and Wascana Creek. The meandering Qu'Appelle River is joined by Last Mountain Creek, then passes through Pasqua, Echo, Mission and Katepwa Lakes and further downstream Crooked and Round Lakes before it crosses into Manitoba and joins the Assiniboine River near St. Lazare.


The most general physical characteristic of the basin is a flat to gently undulating and generally treeless plain. The elevation of this plain ranges from 580 m above sea level at the western end, to 480 m above sea level at the eastern end, with the local relief generally not exceeding 3 m.


In contrast, the most striking feature of the basin is the Qu'Appelle Valley itself. Through this valley, a former glacial spillway which traverses the entire length of the basin, flows the Qu'Appelle River, which meanders along the valley bottom. The valley is incised 30 to 100 m into the plain; it has steep side slopes and a relatively flat bottom which varies from 1.0 to 3.0 km in width.

The climate of the Qu'Appelle River Basin is characterized by hot summers, cold winters, and generally moderate precipitation. The average annual precipitation ranges from 350 mm in the western portion of the basin to 500 mm in the eastern portion. Snowfall makes up about 25 percent of the total annual precipitation (Hydrological Atlas of Canada, Environment Canada, 1978). The basin experiences a mean annual gross evaporation of approximately 900 mm (PFRA Hydrology Report #121, 1989).



LEGEND


Control Structure 

Modified Channel 



No.	Date	Revision	By	App'd

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SASKATCHEWAN WATER CORPORATION 

Qu'APPELLE RIVER BASIN

Figure 1.2-1

Drawn by	Date	Checked by	Date
Approved by	Date		

PLAN DATE

Plan No. page 4

LOCATION PLAN

The drainage area contributing directly to the Qu'Appelle River alone is relatively small when compared to the combined drainage areas of all the tributaries; hence, the hydrology of the Qu'Appelle River is dominated by the runoff characteristics of the tributaries. Since most of the surface water in the basin originates from snowmelt, the main characteristics of the tributary streams is high volumes of flow during the spring runoff period with little or no flow during the remainder of the year.

Approximately 90 percent of the total volume of water contributed to the Qu'Appelle River by its tributaries occurs during the period of March to May. Flows in the Qu'Appelle River for the remainder of the year generally represent a draining of the system with only minor contributions from rainfall events. Also, there are significant known groundwater contributions to Last Mountain Lake and the Fishing Lakes in preserving water levels and maintaining base flows.

1.4 Man-Made Modifications to Qu'Appelle River Basin

The high seasonal and annual variation in water supply in the Qu'Appelle River Basin has prompted man to construct numerous water control structures in the basin. These structures are used to regulate flows and water levels, and therefore alter the natural flow. Table 1.4-1 lists the types of man-made structures in the Qu'Appelle River Basin which alter the natural flow of water in the basin.

TABLE 1.4-1

TYPES OF MAN-MADE MODIFICATIONS AND DEVELOPMENTS IN THE QU'APPELLE RIVER BASIN WHICH ALTER NATURAL FLOW

- (i) Adjustable Lake Outlet Structures
- (ii) Variable Diversion of Water From South Saskatchewan River Basin
- (iii) Numerous Dams and Diversions Within the Basin*
- (iv) Municipal Demands and Return Flows
- (v) Conveyance Works on Main Channel
- (vi) Changes In Land Use and Vegetative Cover
- (vii) Agricultural Drainage
- (viii) Urban Development

* Note: Licensed diversions in the Qu'Appelle River Basin as of December 31, 1988 totalled 79900 dam³.

1.5 Qu'Appelle River Natural Flow Model

In the early 1970s, the Prairie Provinces Water Board acquired the services of Environment Canada to develop the Qu'Appelle River Natural Flow Model. The Qu'Appelle River Natural Flow Model makes use of the SSARR Model. The SSARR model, developed by the United States Army Corps of Engineers, was chosen because it is capable of simulating upstream releases and diversions and streamflow from upstream to downstream points considering channel and lake storage, overbank flow, and backwater effects at lake outlets. Details on the SSARR Model can be found in the User Manual - SSARR Model, Streamflow Synthesis and Reservoir Regulation, U.S. Army Corps of Engineers, August 1987.

A detailed description of the natural flow computational procedure used in the Qu'Appelle River Natural Flow Model can be found in the two-volume

report "Natural Flow - Qu'Appelle River at Saskatchewan/Manitoba Boundary" published by the Prairie Provinces Water Board in December 1975.

Specific details on the model should be obtained from these reports. However, in brief, the Qu'Appelle River Natural Flow model has two segments: a fall and winter segment and a spring and summer segment. The fall and winter segment is used to simulate the natural drawdown of the lakes in the Qu'Appelle River Basin over the August 1 to March 1 period. The spring and summer segment of the model is used to simulate the natural spring runoff over the March 1 to July 31 period. The spring and summer segment of the model has two simulation limbs; a natural conditions limb and an existing conditions limb. In the natural conditions limb, natural streamflows and lake elevations are simulated. The existing conditions limb is used to estimate local inflow for input into the natural conditions limb. The configuration charts for the original model, taken from the original report "Natural Flow - Qu'Appelle River at Saskatchewan-Manitoba Boundary" 1975 are included in Appendix B.

1.6 Deficiencies of Original Model

Since the model was developed in 1975, the Qu'Appelle River Natural Flow Model has been used to estimate the flow which would have occurred in the Qu'Appelle River under natural conditions. However, soon after use of the model commenced, numerous problems with the model were evident.

In 1980, the Prairie Provinces Water Board's Committee on Hydrology formed a sub-committee to study what modifications needed to be made to the Qu'Appelle River Natural Flow Model. This Qu'Appelle SSARR Model Sub-Committee held numerous meetings over the period of 1980 to 1988. The sub-committee studied model characteristics and simulation capabilities and identified model deficiencies that needed to be addressed in order to make the natural flow and "existing conditions" simulations more realistic and, therefore, acceptable.

In 1987, the Water Resources Branch of Environment Canada prepared a project description and cost estimate for modifying the Qu'Appelle River Natural Flow Model. It was estimated that 30 person-months would be required to modify the Qu'Appelle River Natural Flow Model. At the following Qu'Appelle SSARR Model Sub-Committee meeting held on March 3, 1988, the sub-committee identified three options which were presented to the Committee on Hydrology. The three options identified are described in Table 1.6-1.

TABLE 1.6-1

THREE OPTIONS IDENTIFIED BY THE QU'APPELLE SSARR MODEL SUB-COMMITTEE

Option 1: Do nothing.

Option 2: Undertake detailed calibration work as proposed by Environment Canada.

Option 3: Modify the existing model with emphasis on the following work components:

- (a) Extend the stage-area and stage-capacity curves of Last Mountain Lake further down to cover the entire range of lake levels and incorporate extended curves into the existing model;
- (b) Revise the routing parameters for channel reaches under the post-conveyance condition;
- (c) Implement the reservoir regulation cards and modify the evaporation (4P) cards of the existing model;
- (d) Revise the procedure used for computing lateral inflow by utilizing an effective drainage area ratio concept; and
- (e) Evaluate the effect of groundwater on Last Mountain Lake and the Fishing Lakes, and incorporate these effects into the existing model.

The SSARR Sub-Committee recommended that Option 3 be considered by the Committee on Hydrology.

At Committee on Hydrology Meeting #56 held in March 1988, the committee agreed that Hydrology Service, Saskatchewan Water Corporation should provide a study proposal, including a cost estimate to complete the five work items listed in Option 3 as suggested by the SSARR Sub-Committee (COH Minute 56-53).

1.7 Memorandum of Understanding

The Saskatchewan Water Corporation provided the Committee on Hydrology with a study proposal, and in April 1989, a Memorandum of Understanding was signed by the Prairie Provinces Water Board and the Saskatchewan Water Corporation. This Memorandum of Understanding can be found in Appendix A.

1.8 Units

At the December 14, 1987 Qu'Appelle SSARR Model Sub-Committee meeting, it was determined that it would not be cost-effective to convert the Qu'Appelle River Natural Flow Model to SI Units. As a result of this decision, the conversion of the model to metric units was not included in the list of work required for this report. It is for this reason that the discussions and results of this study are reported in Imperial Units. Metric equivalents are provided in parenthesis.

CHAPTER II

DISCUSSION OF MODEL MODIFICATIONS

Schedule A of the Memorandum of Understanding (found in Appendix A) lists the modifications to be made to the Qu'Appelle River Natural Flow Model. Sections 2.1 to 2.9 provide a discussion of the modifications made to the model. Configuration charts of the modified model are shown in Figures 2-1 to 2-10. The modifications made to the model are highlighted on these figures with enhanced lines. Sample input files of the modified model are found in Appendix D.

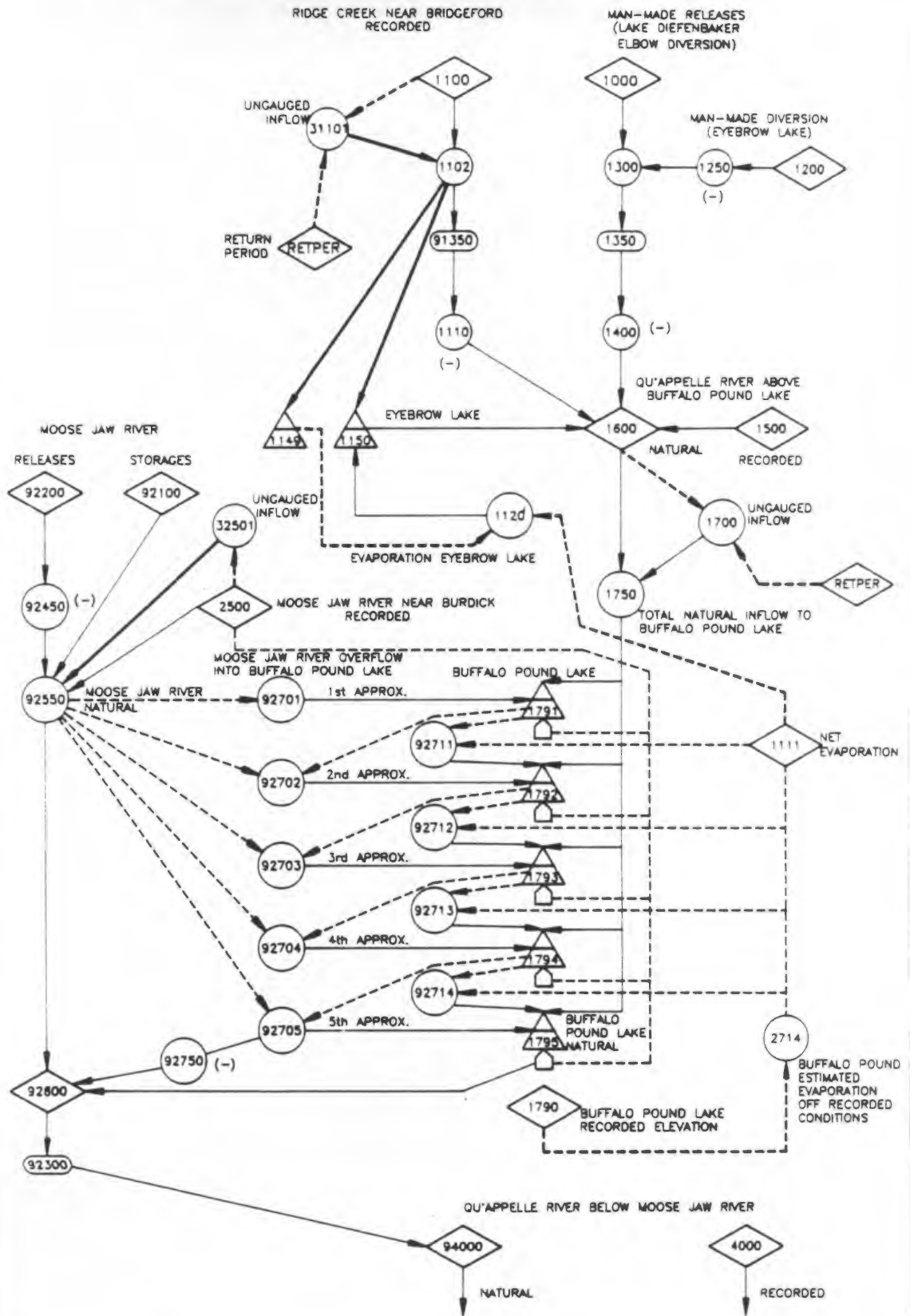
2.1 Implement the Existing Qu'Appelle River Natural Flow Model on a Microcomputer

2.1.1 Installation of SSARR Model on Microcomputer

The microcomputer version of the Streamflow Synthesis and Reservoir Regulation (SSARR) model was obtained from the United States Corps of Engineers, North Pacific Division. The August 1987 version of the model was obtained at a cost of \$60.00 (U.S. Funds).

Included with the SSARR Model, which was provided on seven 5-1/4" floppy disks, was a copy of the SSARR User Manual. Also included was a paper discussing the microcomputer version of SSARR and its installation. The SSARR microcomputer version was installed on an

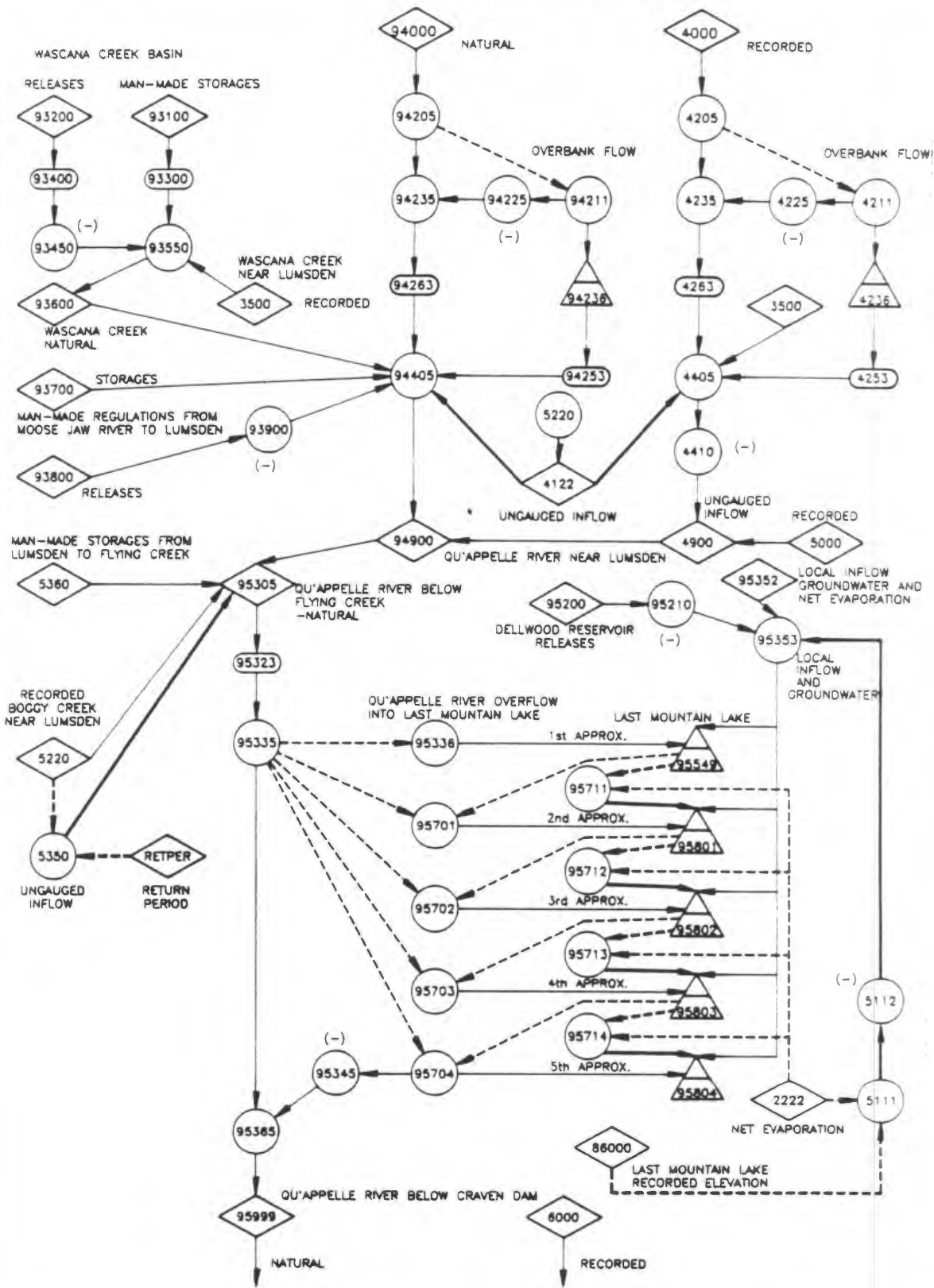
SPRING AND SUMMER MODEL



Modified Model Configuration Chart (Q12) -Lake Diefenbaker to below Moose Jaw River

Figure 2-1

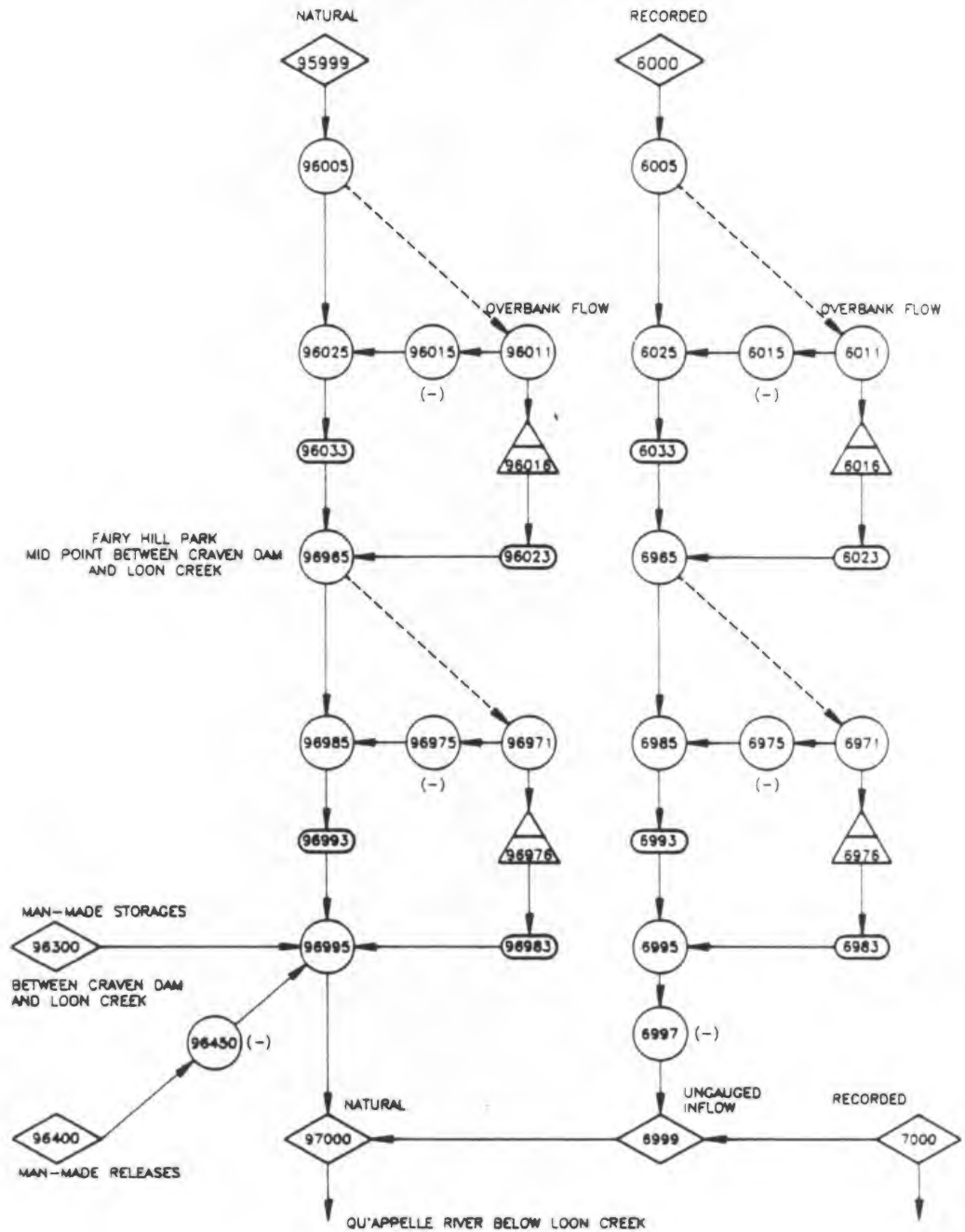
QU'APPELLE RIVER BELOW MOOSE JAW RIVER



Modified Model Configuration Chart (Q12) - below Moose Jaw River to Craven Dam

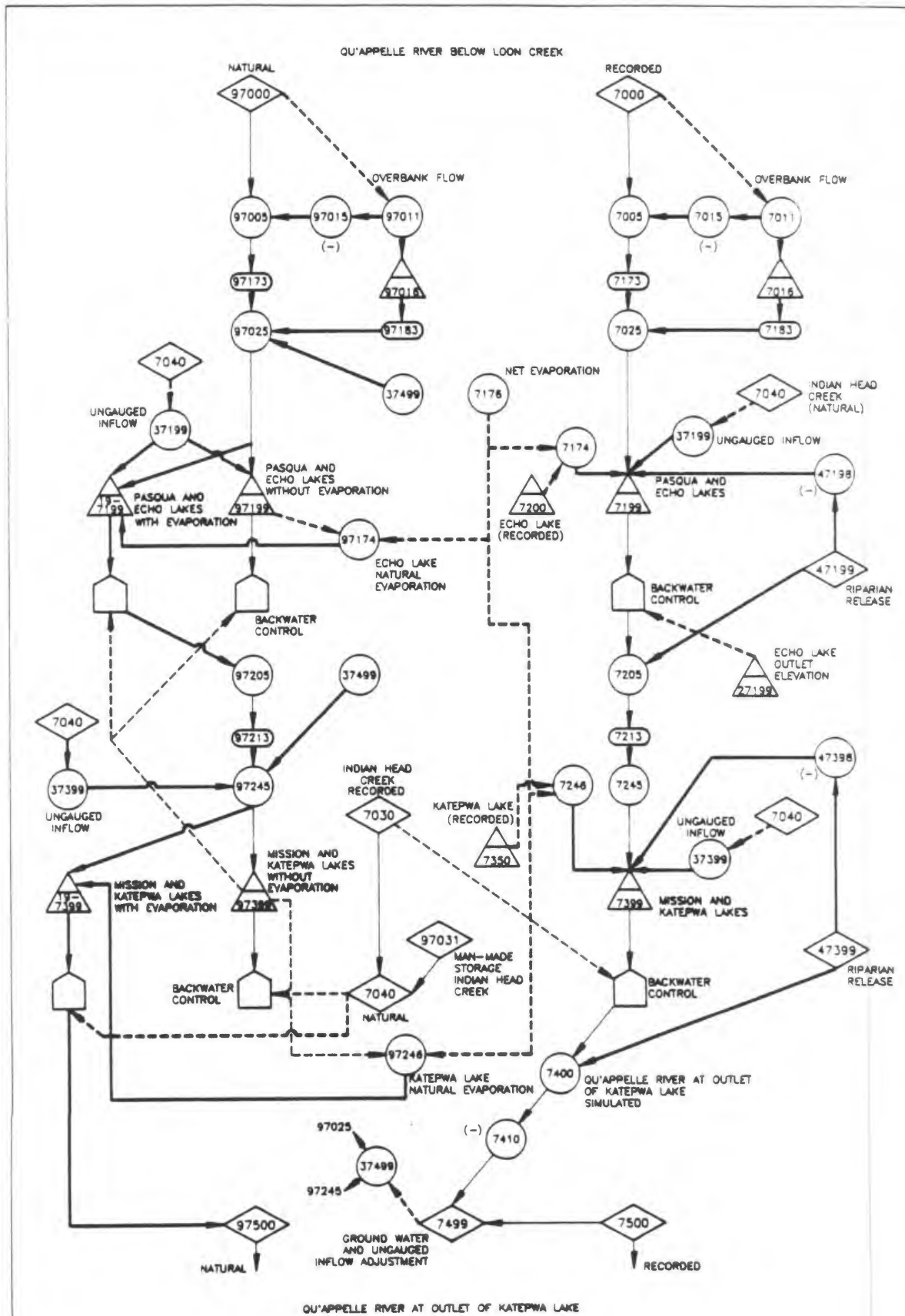
Figure 2-2

QU'APPELLE RIVER BELOW CRAVEN DAM



Modified Model Configuration Chart (Q12) - below Craven Dam to below Loon Creek

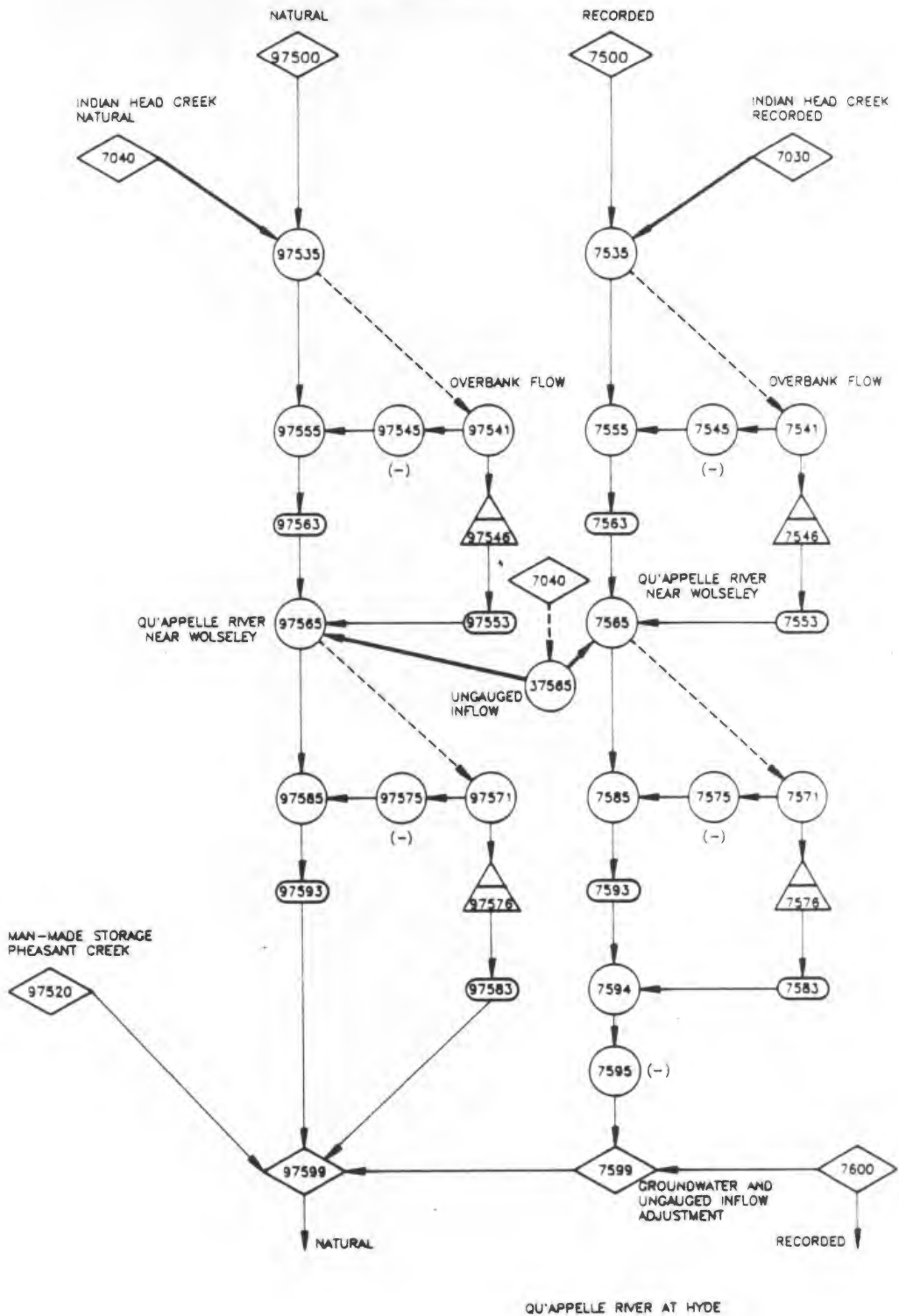
Figure 2-3



Modified Model Configuration Chart(Q12)-below Loon Creek to below Katepwa Lake

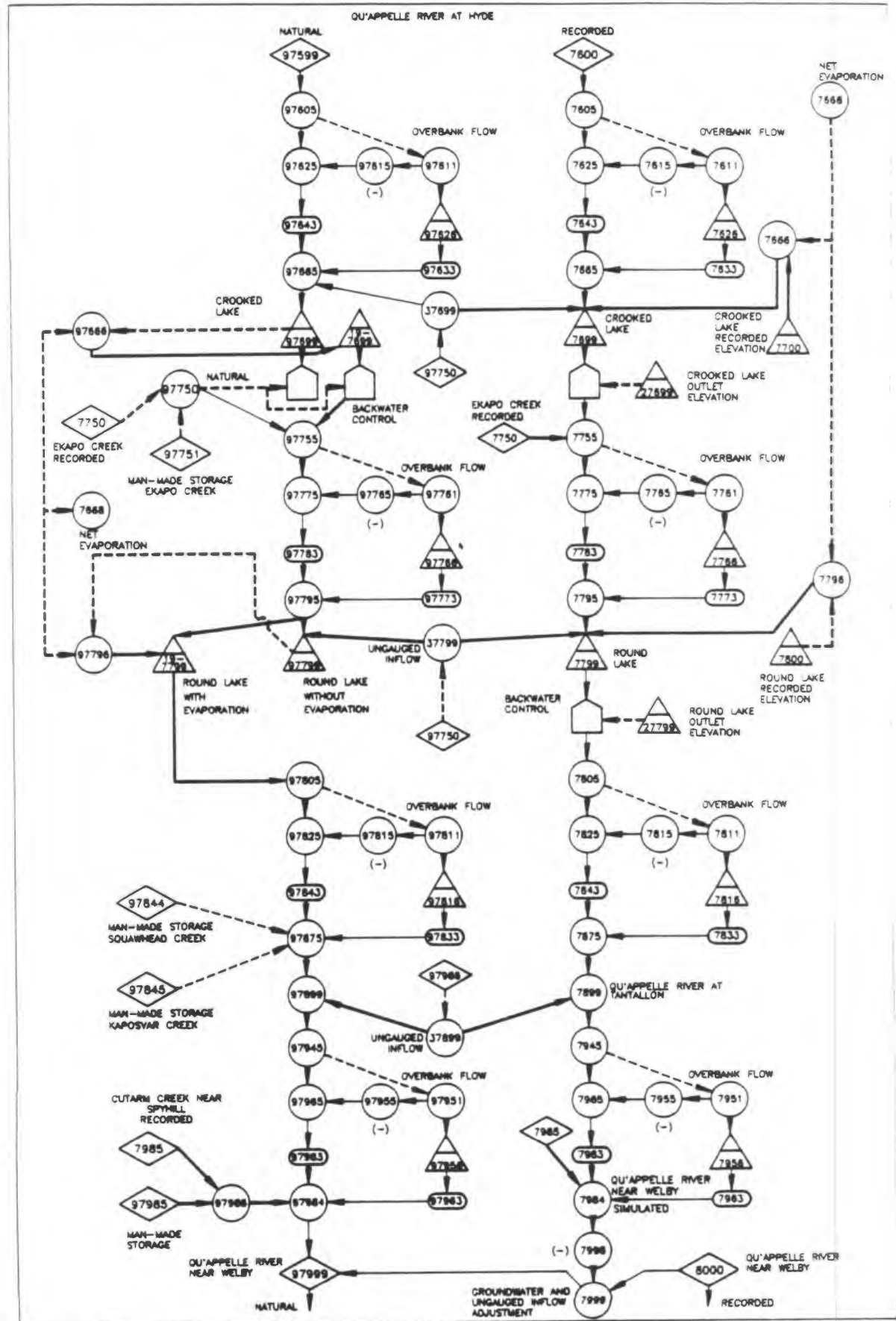
Figure 2-4

QU'APPELLE RIVER AT OUTLET OF KATEPWA LAKE



Modified Model Configuration Chart (Q12) - below Katepwa Lake to Qu'Appelle River at Hyde

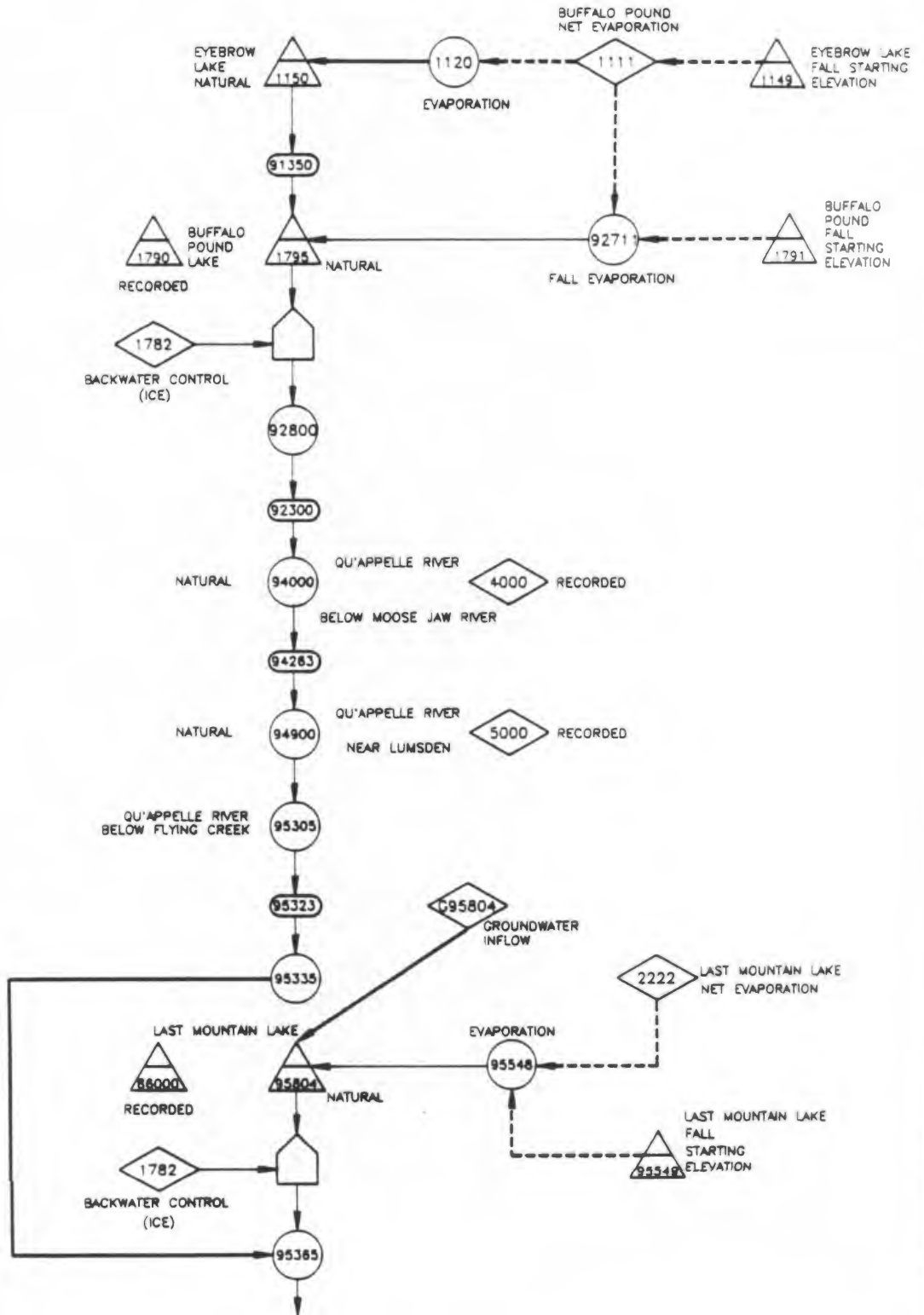
Figure 2-5



Modified Model Configuration Chart (Q12) - Hyde to Welby

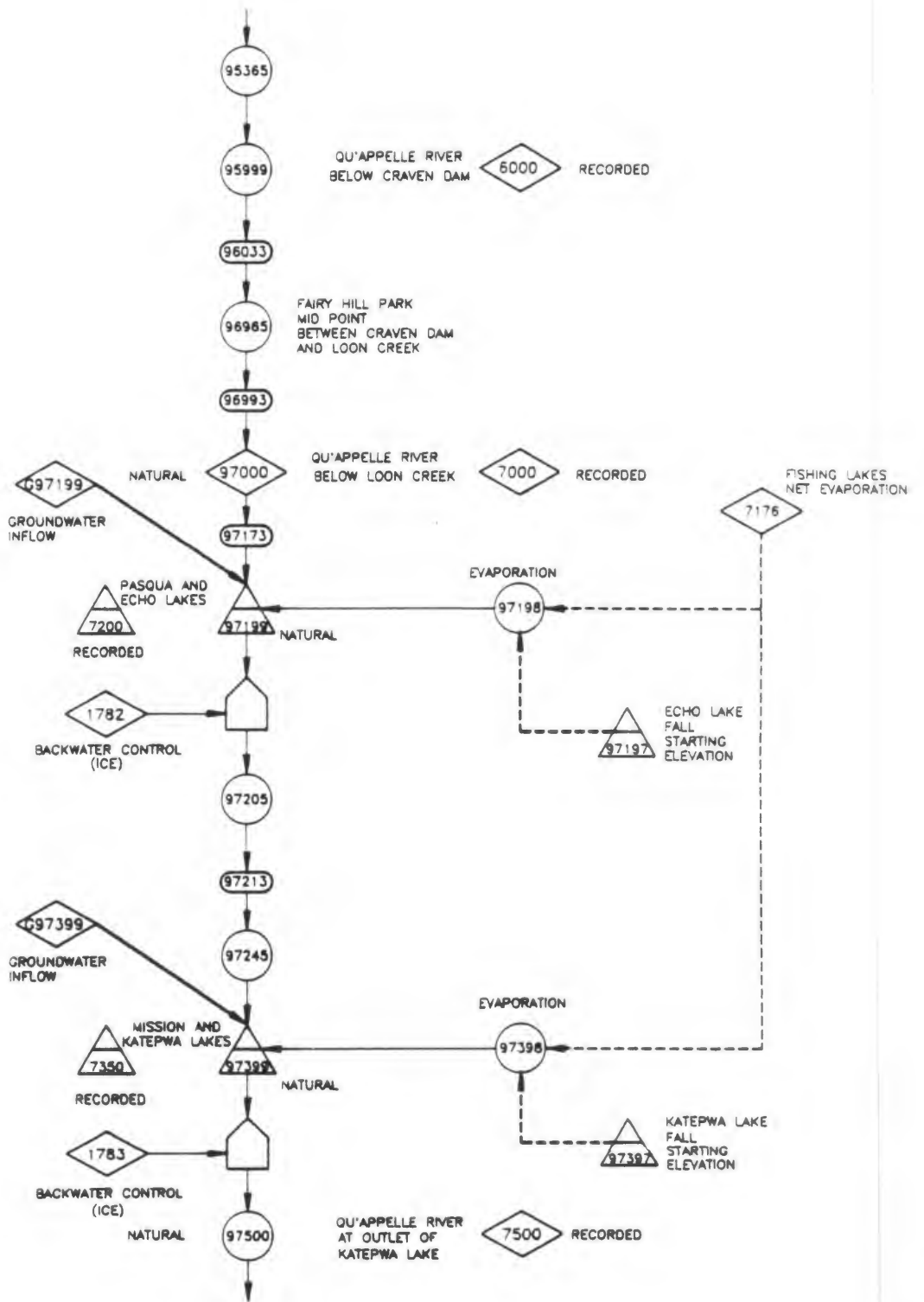
Figure 2-6

Fall and Winter Model



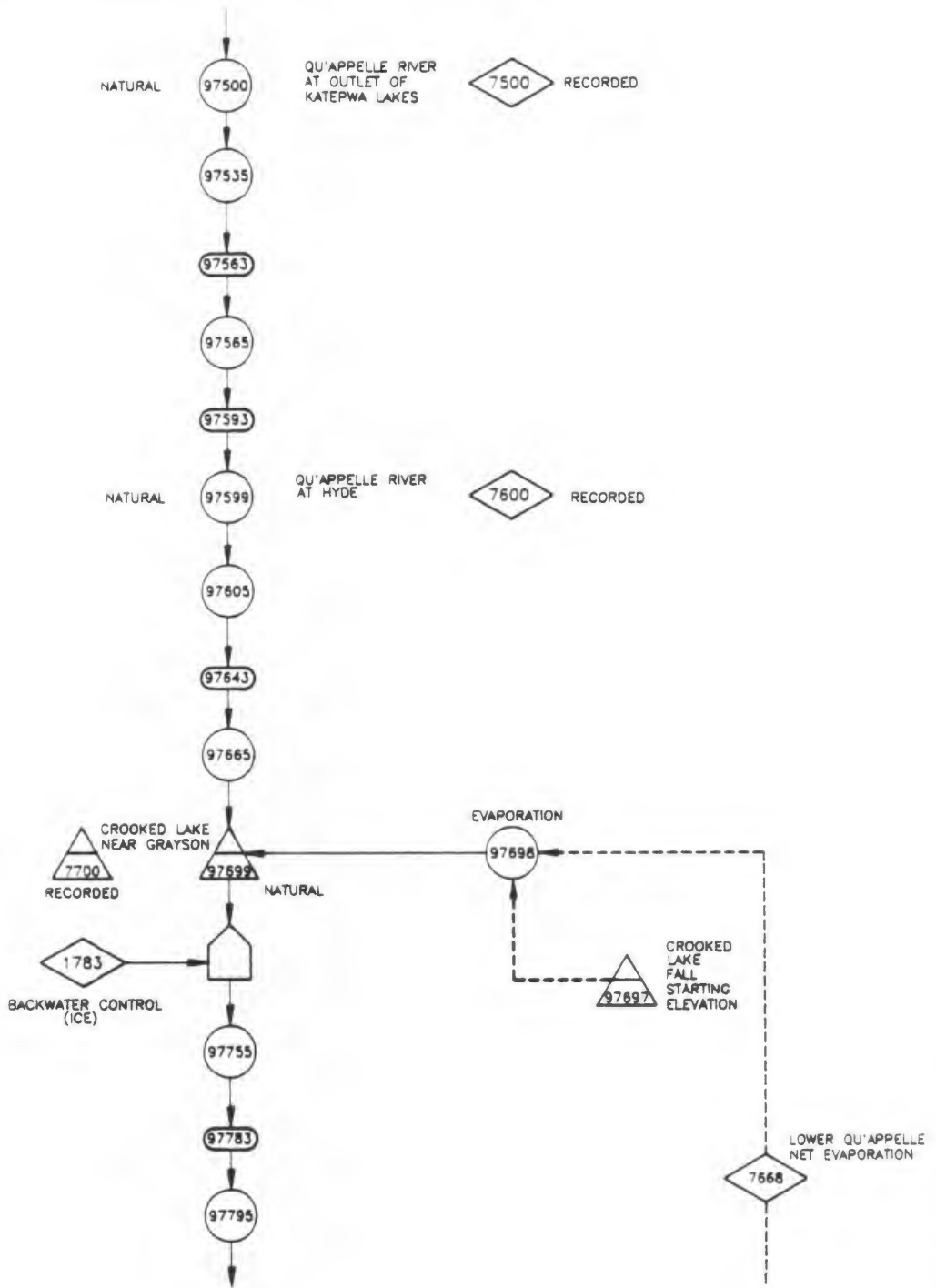
Modified Model Configuration Chart (Q3) - Eyebrow Lake to below Craven Dam

Figure 2-7



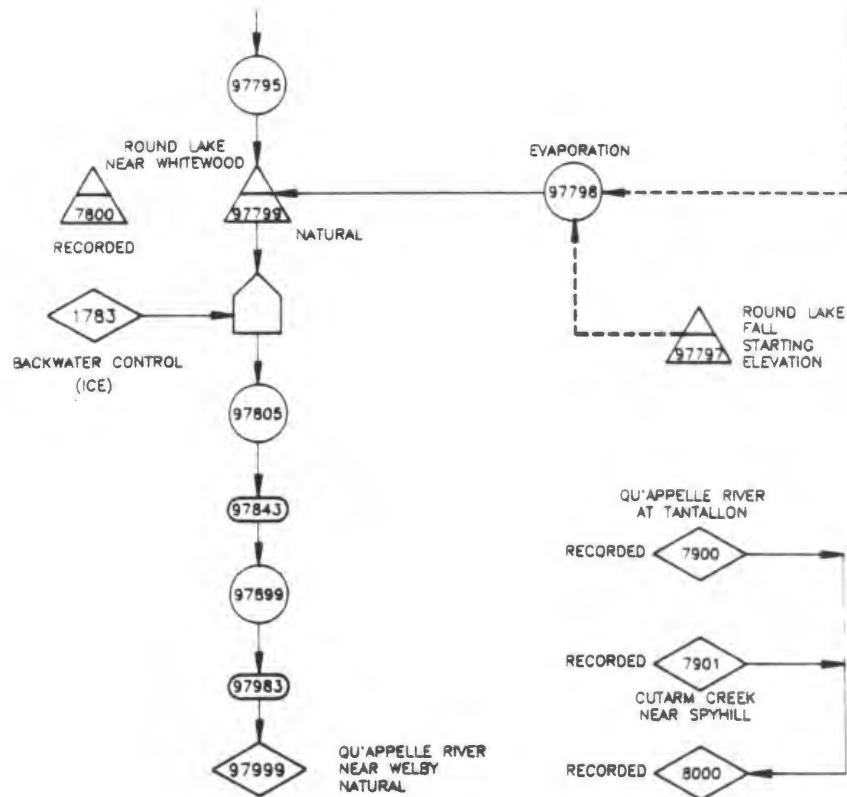
Modified Model Configuration Chart (Q3) - below Craven Dam to below Katepwa Lake

Figure 2-8



Modified Model Configuration Chart (Q3) - below Katepwa Lake at above Round Lake

Figure 2-9



Modified Model Configuration Chart (Q3) - above Round Lake to Qu'Appelle River near Welby

Figure 2-10

IBM Model 50 (AT286, 20 meg hard disk, 1 meg RAM) as per the instructions in this paper.

The microcomputer version of the SSARR model was then tested using original Qu'Appelle River Natural Flow Model Input Files set up by the Water Resources Branch of Environment Canada for 1987 natural flow calculations for the Prairie Provinces Water Board.

Output results, obtained using the microcomputer version of SSARR, were compared with results obtained using the mainframe computer version at Westbridge Computers Ltd. (formerly SaskComp). Output results obtained with the microcomputer version and with the mainframe version are plotted in Figures 2.1.2-1, -2 and -3. These figures show the output from the micro and mainframe versions of SSARR to be very similar, however, not identical. The discrepancy is attributed to the difference in versions. The 1975 mainframe version of SSARR is installed at Westbridge. Numerous changes have been made to the SSARR program since 1975, all of which have been incorporated into the 1987 microcomputer version. The microcomputer output results were then compared with results obtained using the micro-vax version of SSARR on a Micro-Vax II mini-computer. It was found that the output results obtained using the microcomputer version and the Vax version were identical.

2.1.2 Converting Original Qu'Appelle River Natural Flow Model to Microcomputer Version Format

The original Qu'Appelle River Natural Flow Model consisted of three separate input files identified as QUA1, QUA2 and QUA3. QUA1 and QUA2 were used to simulate the upper and lower portions of the Qu'Appelle River Basin over the spring and summer period (March 1 to July 31). QUA3 was used to simulate the entire Qu'Appelle River Basin over the fall and winter period (August 1 to February 28).

The spring and summer segment of the original model was split into two portions to reduce computational costs which were a major concern at the time of original model development in 1975. QUA1 simulated the upper portion of the Qu'Appelle River Basin (from below the Qu'Appelle Dam to below the Craven Dam). QUA2 simulated the lower portion of the Qu'Appelle River Basin (from below the Craven Dam to the Saskatchewan/Manitoba Boundary near Welby). With the conversion of the Qu'Appelle River Natural Flow Model to the microcomputer, the need to minimize computer time is no longer a major concern. QUA1 and QUA2 were therefore combined into one input file.

The combined spring and summer input file, referred to in this report as Q12, runs in approximately 20 minutes. The fall and winter portion input file, referred to as Q3 in this report, runs in approximately 5 minutes. (Note: Run times were obtained using an IBM AT286 with math co-processor).

**COMPARISON OF MICRO AND MAINFRAME VERSIONS
ORIGINAL QU'APPELLE RIVER SSARR NATURAL FLOW MODEL
QU'APPELLE RIVER BELOW CRAVEN DAM NATURAL FLOW (QUA1)**

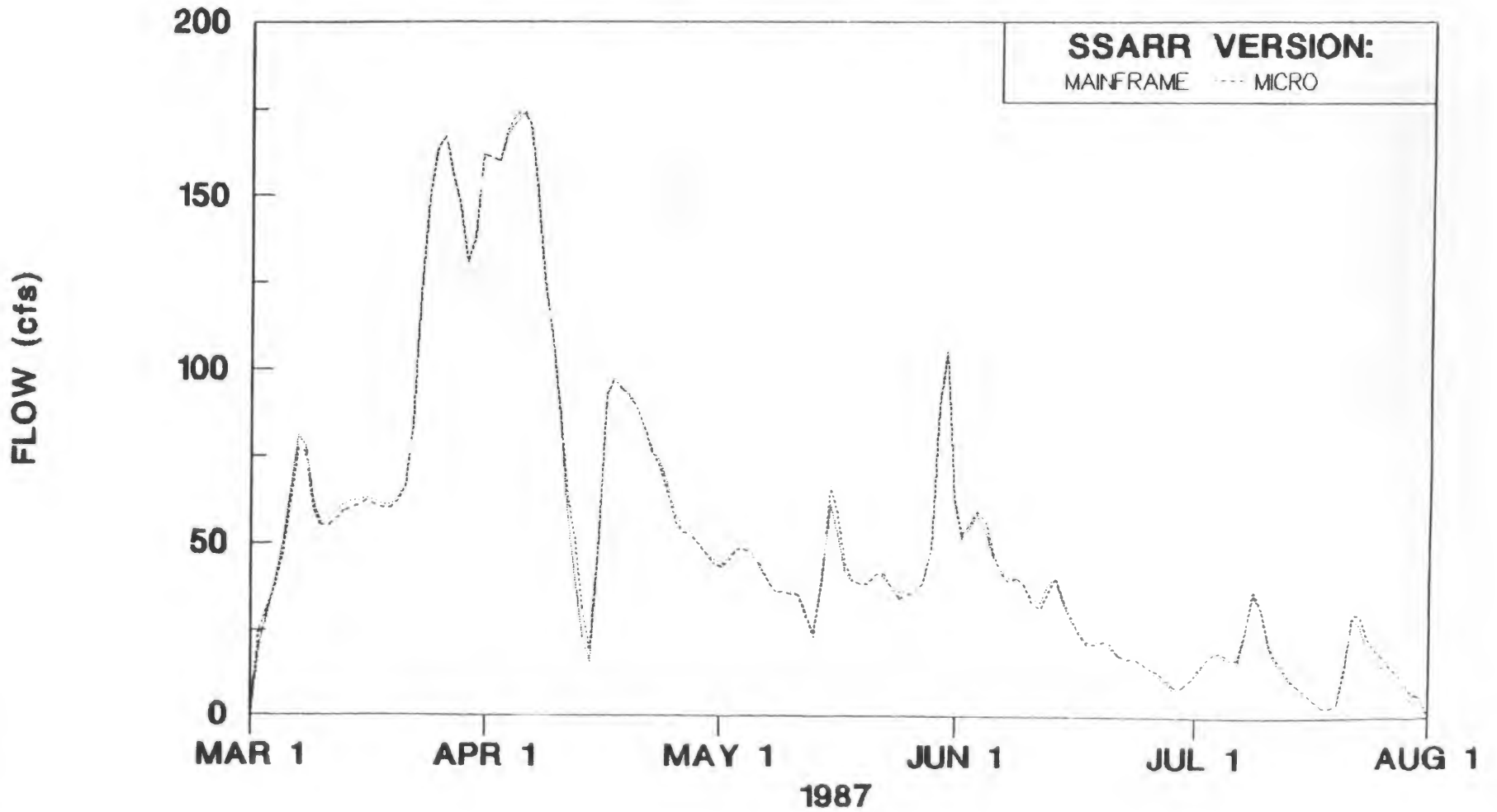


Figure 2.1.2-1

**COMPARISON OF MICRO AND MAINFRAME VERSIONS
ORIGINAL QU'APPELLE RIVER SSARR NATURAL FLOW MODEL
QU'APPELLE RIVER NEAR WELBY SIMULATED NATURAL FLOW (QUA2)**

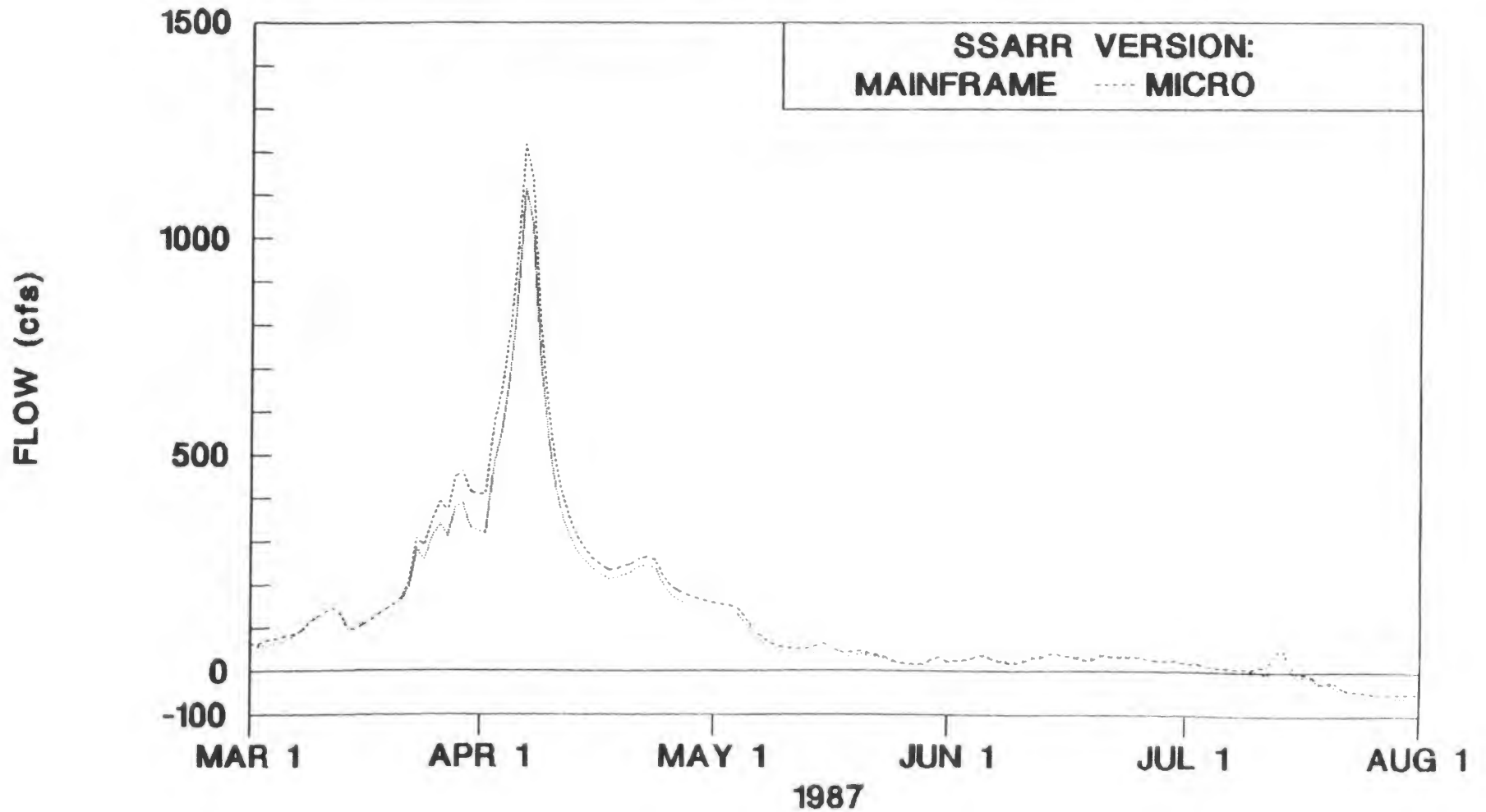


Figure 2.1.1.2-2

**COMPARISON OF MICRO AND MAINFRAME VERSIONS
ORIGINAL QU'APPELLE RIVER SSARR NATURAL FLOW MODEL
LAST MOUNTAIN LAKE SIMULATED NATURAL ELEVATION (QUA3)**

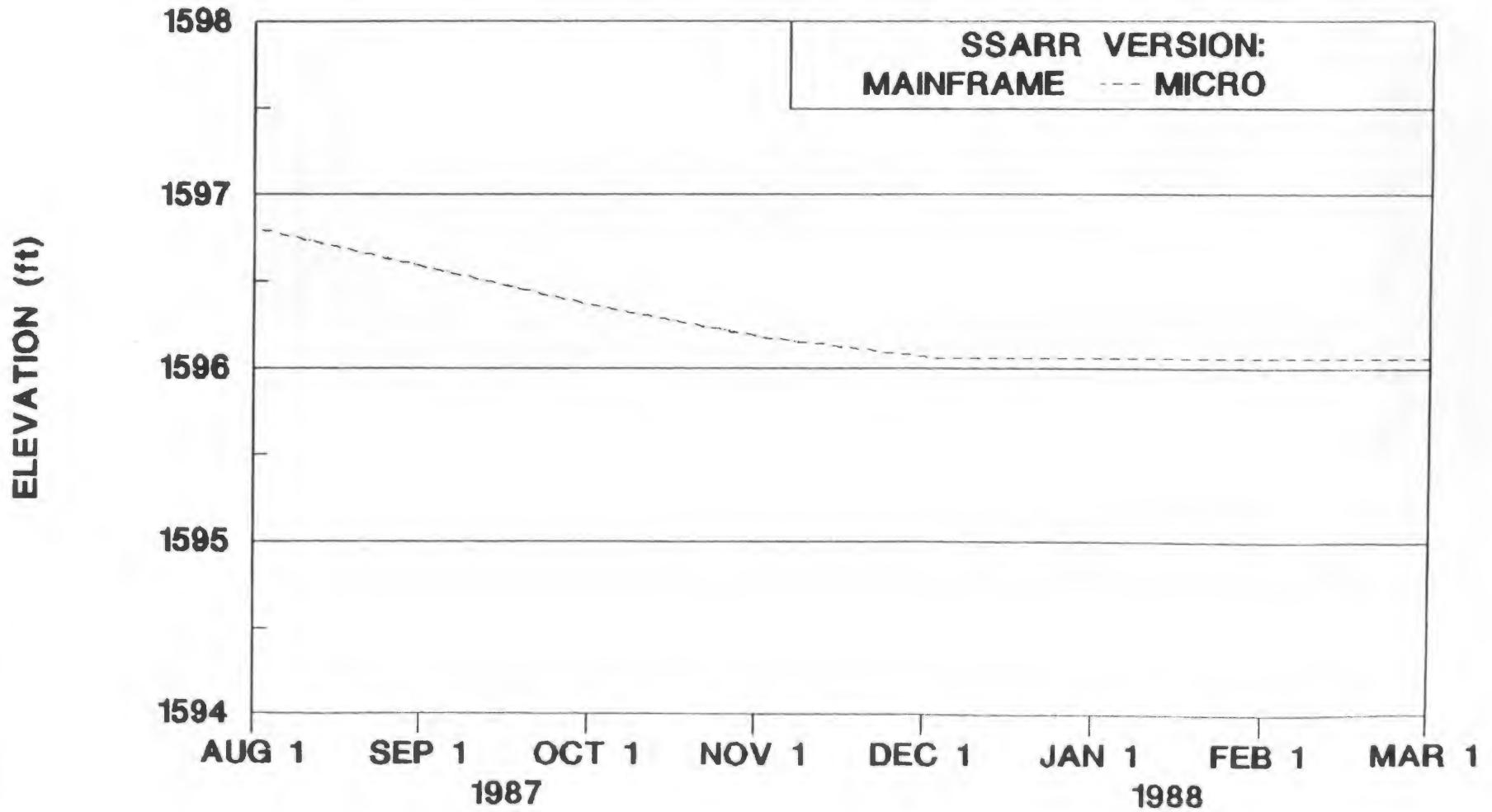


Figure 2.1.2-3

2.2 Extend Last Mountain Lake Elevation-Storage Tables

The original Qu'Appelle River Natural Flow Model was developed in the early to mid 1970s. During this period, the Qu'Appelle River Basin was experiencing high runoff events. It is therefore not surprising that the original model was set up with emphasis on high flows and high water levels. As the Qu'Appelle River Basin began experiencing low runoff years during the 1980s, the need to modify the model for low flows and low water levels became evident.

In the original model, the lowest specified data point in the elevation-storage table (C1 card) was for an elevation of 1,601 feet (487.985 m). The source of this original table was not documented in the manual of the original model. The original table was replaced with elevation-storage table shown in Figure 2.2-1.

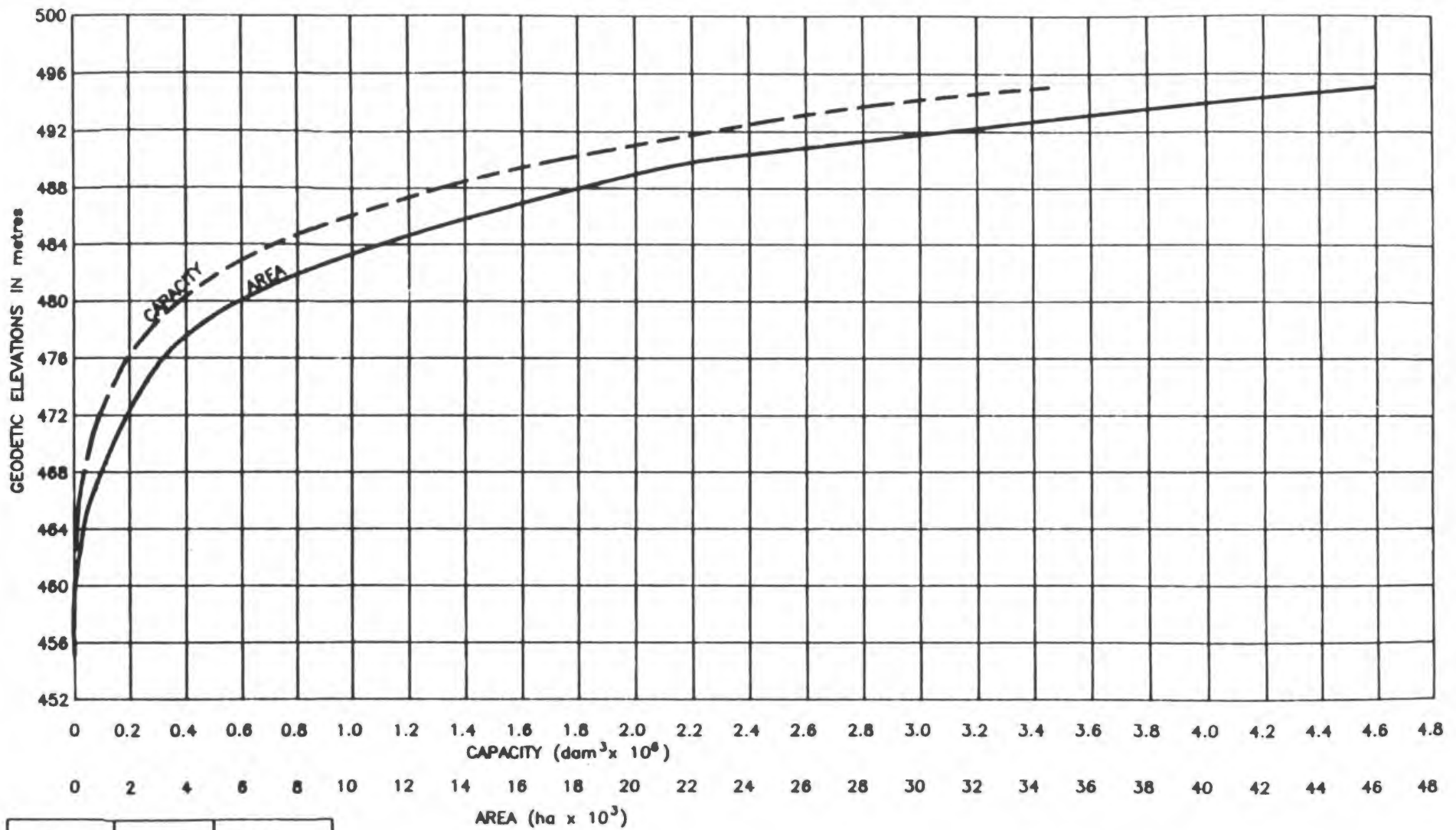
The original three-dimensional table used in the natural flow limb to determine the fraction of flow in the Qu'Appelle River which flows into (or out of) Last Mountain Lake was also set up with emphasis on high flows and high water levels. The original three-dimensional table is plotted in Figure 2.2-1. This figure shows the original table did not go below an elevation of 1,597 feet (486.766 m). Figure 2.2-2 shows that the amount of simulated inflow (or outflow) from Last Mountain Lake during low flows on the Qu'Appelle River must be interpolated between simulated Qu'Appelle River flows of -10 and 500 cfs. Figure 2.2-2 also shows that, at low Qu'Appelle River flows, the original 3D table simulated outflow from Last Mountain Lake starting at lake elevations above 1,597 feet.

In order to develop a three-dimensional table which better simulates the natural flow of water in and out of Last Mountain Lake, a number of old plans of the Valeport Control and Craven Control, filed in the water rights files of Sask Water, area were analyzed. Plan 1175-2-A-7 shows the natural outlet elevation of Last Mountain Lake to be 1,605 feet (489.204 m). Thus, under natural conditions, flow out of Last Mountain Lake would not occur at lake elevations below 1,605 feet. Plan 801-P3-1087 shows the natural elevation of the streambed at the confluence of the Qu'Appelle River and Last Mountain Creek to be 1,604 feet (488.899 m). Using Manning's equation, cross sections and streambed slopes obtained from the plans, and Manning's n of 0.07 (minor stream on plains with sluggish reaches and weedy pools), it was determined that under natural conditions, when the elevation of Last Mountain Lake is below 1,605 feet (489.204 m), flows below 25 cfs (0.708 m^3/s) in the Qu'Appelle River would not back up Last Mountain Creek high enough to flow into Last Mountain Lake.

The modified three-dimensional table, which was incorporated into the Qu'Appelle River Natural Flow Model, is shown graphically in Figure 2.2-3. The modified three-dimensional table can be found in the example input file located in Appendix D.

2.3 Revise Channel Routing Parameters

Since the Qu'Appelle River Natural Flow Model was developed, extensive channel improvement work has been carried out on the main stem of the Qu'Appelle River. Although channel modification work has been completed at numerous locations along the length of the Qu'Appelle River as shown in



ELEV. (metres)	AREA (ha)	CAPACITY (dam ³)
455.12	0	0
460.12	25	417
465.12	437	11967
470.12	1433	58717
475.12	2859	166017
480.12	6035	388367
485.12	12787	858917
490.12	22885	1750717
492.72	34000	2461780
495.12	46000	3461780

SOURCE OF DATA:
1951 BATHYMETRIC MAP
PLANIMETERED SEPT 27, 1990

Designed _____
Drawn D.E.S.
Recm'd _____
Approved _____

SASKATCHEWAN
WATER
CORPORATION
Managing a Vital Resource

LAST MOUNTAIN LAKE
AREA/CAPACITY CURVE

Date 90.10.09

Sheet _____ of _____

PLAN No. _____

FIGURE 2.2-1

LAST MOUNTAIN LAKE OVERFLOW TABLE

ORIGINAL TABLE (CF 20)

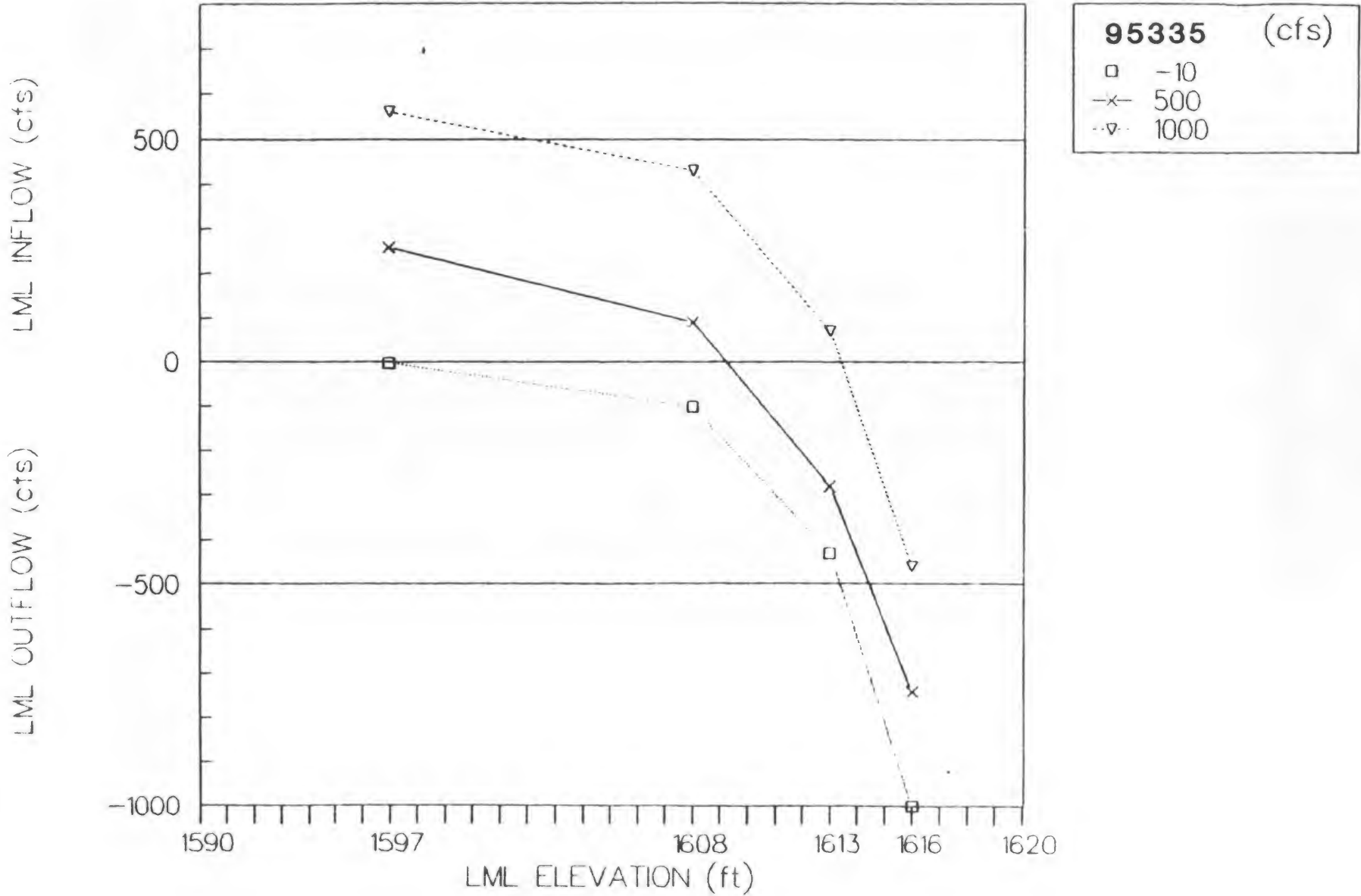


Figure 2.2-2

LAST MOUNTAIN LAKE OVERFLOW TABLE

MODIFIED TABLE (CF 20)

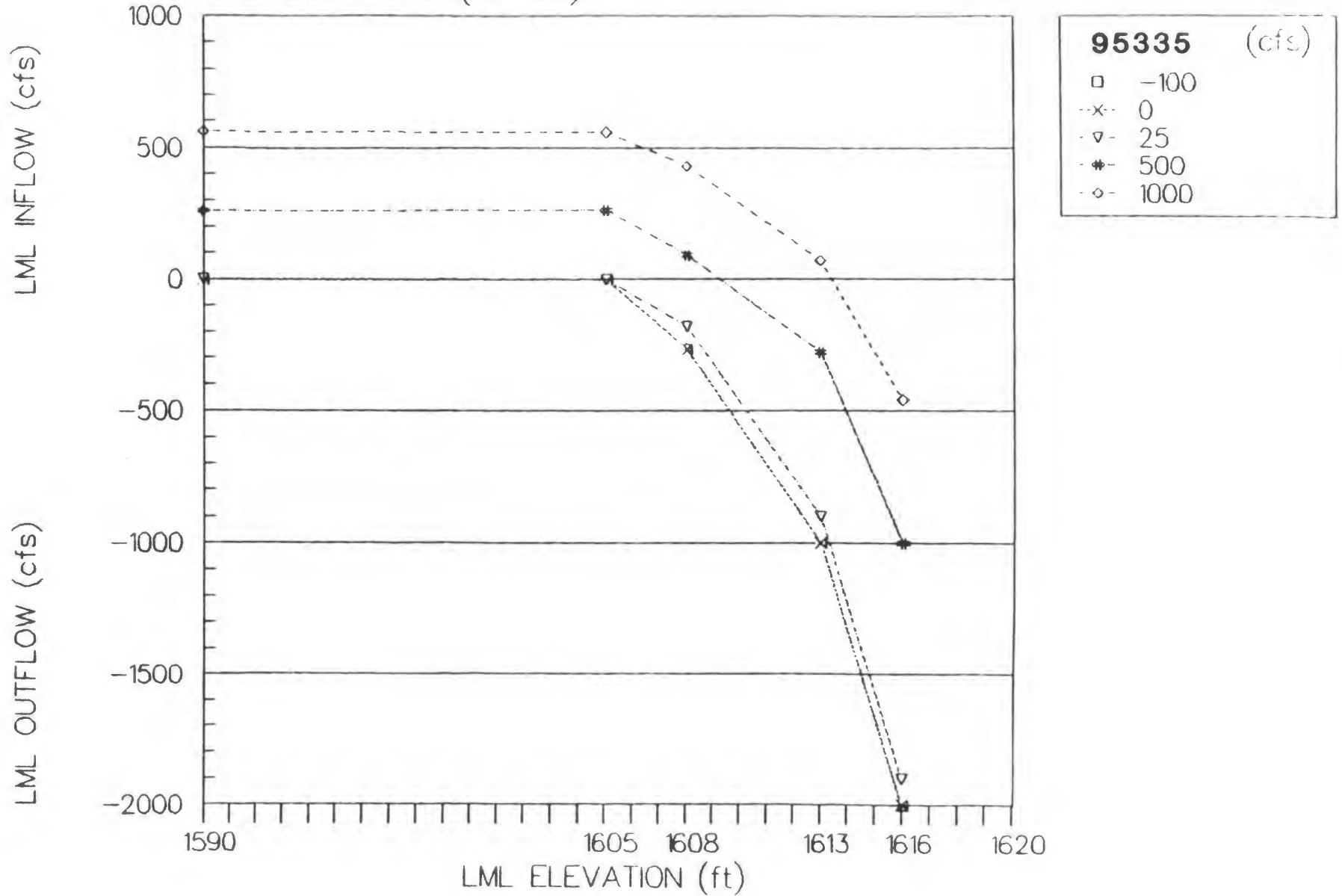


Figure 2.2-3

Figure 1.2-1, the only locations where works are considered to be significant enough to be incorporated into the natural flow model are in the reaches from No. 6 Highway to Pasqua Lake and immediately below Katepwa Lake. In these reaches, conveyance works completed over the period of 1980 to 1988 have significantly altered both the time of travel and overbank flow characteristics of the Qu'Appelle River.

2.3.1 Streamflow Routing

The SSARR model accounts for streamflow routing by first dividing the reach into a series of increments as specified in the CR02 record. Outflow from each increment is used as inflow to the next downstream increment. The step-by-step procedure is thus completed for each increment for each time period. The relation between time of storage and flow rate is expressed in equation (1):

$$TS = KTS/Q^n \quad (1)$$

where TS = the time of storage per increment (hrs)

TKS = constant for the reach

Q = flow rate (cfs)

n = a coefficient between -1 and 1

The values for KTS and n are specified in the input file in CR02 records. In estimating the effect of constructing loop cutoffs and streamflow time of travel, it was assumed the time of travel was reduced by the same proportion as the resulting reduction of stream length. This assumption does not account for any increase in velocity

due to the increase in channel slope; however, because time of travel is not a crucial factor in determining the natural flow volume, it was determined to be sufficient.

Using travel times obtained from the Report on Stream Time of Travel, Upper Qu'Appelle River (Water Survey of Canada, 1973) new routing parameters for the reaches between Highway No. 6 and Pasqua Lake and below Katepwa Lake were estimated. These new parameters are shown in Table 2.3.1-1.

TABLE 2.3.1-1

ROUTING PARAMETERS FOR
PRE- AND POST-CONVEYANCE CONDITIONS

Channel Characteristic	Hwy #6 to Loon Creek		Loon Creek to Pasqua Lake		Below Katepwa Lake	
	Pre-1988	Post-1988	Pre-1987	Post-1987	Pre-1980	Post-1980
Channel Length *(km)	27.1	20.8	29.5	19.3	29.5	27.0
Channel Capacity**(cfs)	150	500	300	500	150	500
No. SSARR Routing Segments (n)	5	5	2	2	8	8
KTS	204	157	255	167	64.6	59
n	0.491	0.491	0.553	0.553	0.350	0.350
Overbank Flow CT Table	34	37	32	37	34	37

* Source: Qu'Appelle River Conveyance Project, Post Construction Plans, February 1988

** Source: Qu'Appelle River System and Operation, Technical Document, Qu'Appelle Valley Management Board, 1977

2.3.2 Overbank Flow

In analyzing and checking the new routing parameters, it was discovered that the original method of handling overbank flow does not work as originally intended. The streamflow is split into channel flow and overbank flow as specified in the CT records. However, because no outflow is specified in the elevation/storage/outflow relationship (Cl records) for the overbank reservoirs, the entire overbank flow is passed directly through the overbank reservoir and added back to the channel flow. Thus, the original overbank flow mechanism had no net effect on simulated flows.

The elevation/storage/outflow relationship (Cl records) table for each existing overbank reservoir, and for the new overbank reservoir which was incorporated into the model for the reach from below Loon Creek to Pasqua Lake, were modified in the following manner. Total simulated flow is split into channel and overbank flow as specified in the CT records. The overbank flow would flow into the overbank reservoir, increasing its storage. The elevation of the overbank reservoir would thus increase and outflow from the overbank reservoir would occur as specified in the elevation/outflow table. As the total streamflow recedes to the point where the main channel is capable of handling the entire flow, inflow into the overbank reservoir ceases, however, outflow from the overbank reservoir continues, dropping the storage to the point where flow out of the reservoir began. The volume of water remaining in the overbank reservoir was set equal to the amount of overbank flow lost to

infiltration which was estimated to be the area of the reach valley bottom multiplied by a depth of infiltration. When estimating water use of backflood operations, the depth of infiltration used by the Saskatchewan Water Corporation ranges from four inches to eight inches. The depth of overbank flow assumed to be lost to infiltration in the Qu'Appelle River Valley due to overbank flow was six inches. The area of valley bottom of each reach was obtained using 1:50000 Energy, Mines and Resources topographic maps. Table 2.3.2-1 summarizes the overbank reservoir characteristics.

TABLE 2.3.2-1		
OVERBANK FLOW RESERVOIR CHARACTERISTICS		
Overbank Reservoir SSARR No.	Reach Valley Bottom Area (acre)	Volume of Infiltration* (acre-feet)
4236	8600	4300
6016	6500	3250
6976	5000	2500
7016	5000	2500
7546	7000	3500
7576	6000	3000
7626	3000	1500
7816	5200	2600
7951	6000	3000

* Based on a 6-inch infiltration depth.

2.4 Reservoir Regulation

In the existing conditions limb of the original spring and summer portion of the Qu'Appelle River Natural Flow Model, all lakes are simulated with the lake outlet structures wide open for the entire period. This is not

a reasonable simulation of the existing conditions as in all but extremely high runoff years, the outlet structures are operated so as to maintain optimum lake elevations and meet downstream demands.

The SSARR model has the ability to simulate reservoir operations. This internal method of reservoir operation simulation forces reservoir elevations to equal specified elevations on specified dates using 6S records. This method of simulating the reservoir regulation was tested to determine if it is suitable for incorporation into the Qu'Appelle River Natural Flow Model.

When using this method of simulating reservoir regulation, the model calculates a lake outflow by completing a water balance on the lake. Using the net lake inflow and the change in lake storage, which is calculated using the change in recorded elevation, the lake outflow is calculated. Unfortunately, due to the size of the lakes in the Qu'Appelle River Valley, even small changes in the recorded elevation indicate relatively large changes in lake storage, resulting in a large simulated lake outflow or inflow.

Thus, even minor anomalies in the recorded lake elevation data caused by wind set-up or any other factor which affects the accuracy and precision of recorded elevation data, have a major effect on the accuracy of the simulated lake outflows. Figure 2.4-1 shows the outflow of Echo Lake simulated using 6D reservoir regulation records. This figure shows the exaggerated oscillation in the simulated lake outflow which results when this method of reservoir regulation is used. Lake outflows simulated using

this method are obviously not a reasonable simulation of actual conditions. Thus, specifying lake elevations using 6D regulation records was not found to be a suitable method of simulating reservoir regulation.

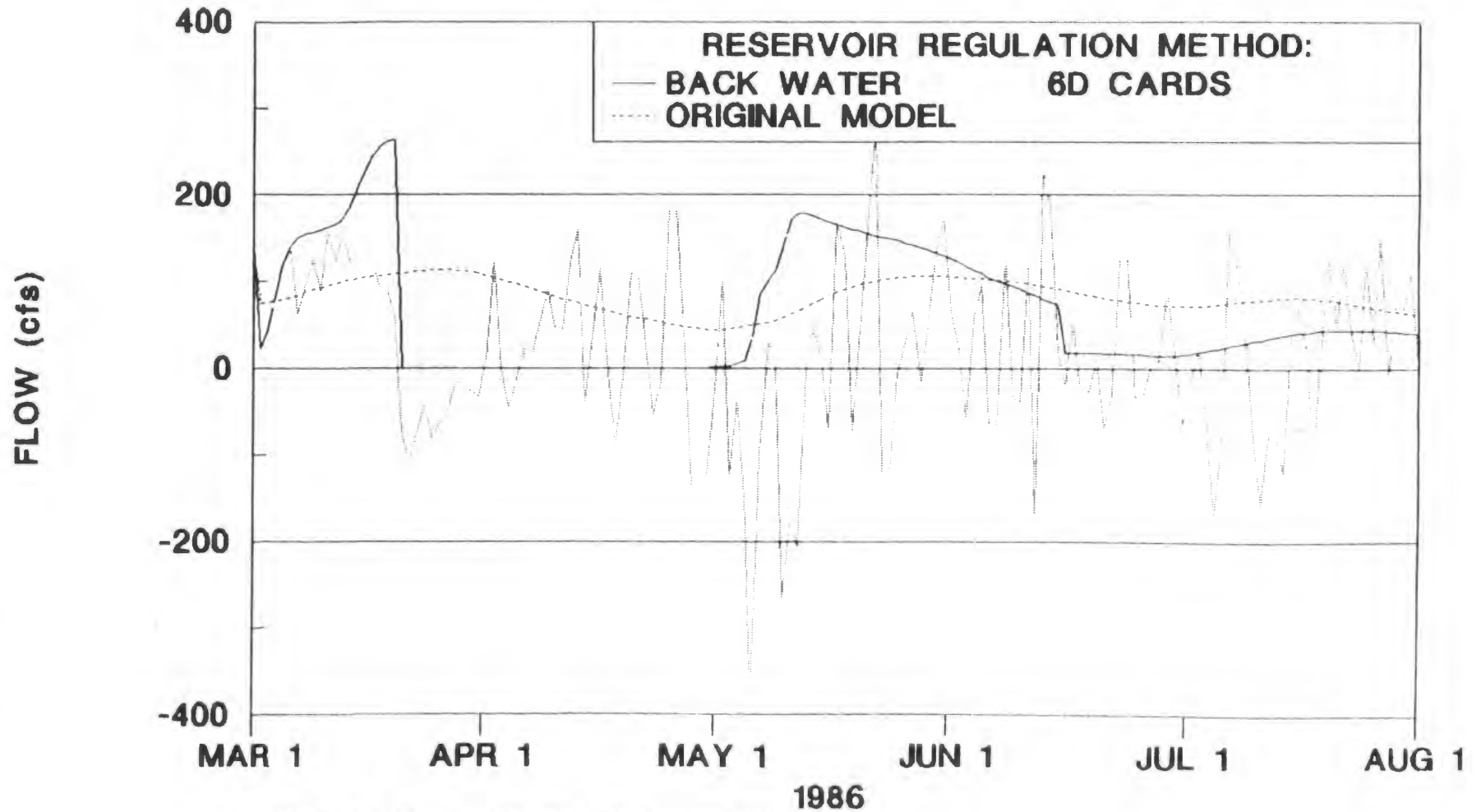
Lake outlet structures operations have the effect of changing the lake elevation/outflow curve. The SSARR model has the ability to alter the elevation/outflow relationship with the use of backwater control records. This method utilizes a three-variable relationship between the lake elevation, the downstream control (either a flow or an elevation) and outflow from the lake.

This backwater control option is used in both the natural and existing conditions limbs to simulate the backwater effect caused by high tributary flows entering immediately below a lake outlet. In the existing conditions limb of the modified model, a backwater effect was used to simulate reservoir regulation. The downstream controlling factor was changed from tributary inflow to the elevation of a dummy reservoir. The elevation of the dummy reservoir was then set equal to the elevation of the outlet structure to simulate outlet structure operations. When the logs in the various bays in an outlet structure are set at differing elevations, the dummy reservoir is set to the elevation of the lowest bay.

The SSARR model allows only one backwater control station so in the case of Echo Lake and Crooked Lake, the backwater effects of Katepwa Lake and Ekapo Creek, respectively, must be simulated using the outlet elevation of the dummy reservoirs.

ECHO LAKE OUTFLOW

COMPARISON OF OUTFLOWS SIMULATED BY EXISTING
CONDITIONS LIMB OF MODEL



1 March 20, 1986 - Stoplogs raised to elevation 157123 ft
2 June 15, 1986 - Stoplogs raised to elevation 157152 ft

Releases made through the gates at the Echo and Katepwa Lake outlets are simulated by adding a flow equal to the actual release to the lake outflow and subtracting it from the respective lake storage. The riparian release, including flow over the fish ladder, must be specified with 6D records.

Figure 2.4-1 shows Echo Lake outflow simulated by the existing conditions limb of the model using the different methods of reservoir regulation. This figure shows the backwater method best simulates the affect of structure operations on reservoir outflow.

The modified model configuration charts in Figures 2-4 and 2-6 (Pages 15 and 17) show how reservoir regulation has been incorporated into the existing conditions limb of the Qu'Appelle River Natural Flow Model.

2.5 Lake Evaporation

The original spring and summer portion of the Qu'Appelle River Natural Flow Model compensated for lake evaporation with a net inflow residual factor on all lakes within the Qu'Appelle River basin (with the exception of Eyebrow Lake and Buffalo Pound Lake). The net inflow residual factor was calculated using the change in lake storage which was calculated using recorded month-end lake elevations. The problem with this method of handling evaporation is that there is no compensation for the increased evaporation due to the present lake surfaces being larger than natural due to higher lake levels caused by the man-made outlet structures.

In the modified natural flow model, lake evaporation is based on the net evaporation estimated for the geographical location of the lake and the lake surface area. The recorded lake elevation is used to estimate the lake surface area on the existing conditions limb of the model. An iterative process is used to determine natural lake elevations used to estimate lake surface area on the natural conditions limb of the model.

Net evaporation for each lake was estimated by subtracting monthly precipitation data recorded at precipitation stations in the vicinity of the lake, from monthly gross evaporation data estimated for the geographic location of the lake. Mean monthly gross evaporation data was estimated by transferring gross evaporation data from Regina to the various lake locations. Gross evaporation data estimated by PFRA using the Meyer Equation was used (PFRA Hydrology Report #121).

Table 2.5-1 lists the precipitation stations, gross evaporation stations and transfer ratios used to estimate the net evaporation at each respective lake in the Qu'Appelle Valley.

TABLE 2.5-1

ESTIMATION OF NET EVAPORATION IN QU'APPELLE RIVER BASIN

Lake	Gross Evaporation Station	Transfer Factor	Precipitation* Stations	SSARR
Eye-brow Lake & Buffalo Pound Lake	Regina	1.0	Buffalo Pound, Marquis, Moose Jaw	1111
Last Mountain Lake	Regina	1.0	Rowan's Ravine, L.M.L. Wildlife, Lumsden, Regina	2222
Fishing Lakes	Regina	0.93	Fort Qu'Appelle, Lebret, Indian Head	7176
Crooked Lake & Round Lake	Regina	0.93	Crooked Lake, Whitehead, Broadview	7668

* Precipitation stations are listed in order of preference.

The configuration charts shown in Figures 2-1, 2-2, 2-4 and 2-6 (Pages 12, 13, 15 and 17) show how evaporation was incorporated into the spring and summer portion of the model.

In the case of Last Mountain Lake, because the net inflow residual contains existing condition evaporation, it is necessary to add back the estimated existing condition evaporation, then subtract the natural conditions evaporation from the natural lake storage.

The method in which evaporation is accounted for in the fall and winter portion of the Qu'Appelle River Natural Flow Model was also modified such that lake evaporation is based on the net evaporation estimated for the geographic location of the lake and the simulated natural lake area of each respective lake. The model configuration charts shown in Figures 2-7, 2-8, 2-9 and 2-10 (Pages 18 to 21) show how lake evaporation was incorporated into the fall and winter portion of the model.

2.6 Local Inflow

The method used by the original Qu'Appelle River Natural Flow Model to compensate for local inflow is described in detail in the report on the original natural flow model. Briefly, the original model estimated ungauged inflow by routing a recorded inflow through a reach. The routed outflow was then subtracted from the recorded outflow. The difference between the routed and recorded outflow was considered to be the net ungauged inflow. The problem with this method is that there was no way to check the accuracy

of the simulation. Elevations and flows simulated with the existing conditions limb could not be meaningfully compared to recorded data because they were simulated without local inflow.

The Qu'Appelle River Natural Flow Model was modified to estimate local ungauged inflow by transferring flows recorded at representative hydrometric streamflow stations using drainage area ratios. This estimated local inflow was included at key locations throughout the model so that simulation results could be compared with actual recorded data. Table 2.6-1 lists the reaches for which local inflow was incorporated. Also shown in this table are the ungauged areas and the recording station which was used to estimate the local inflow and the respective effective and gross drainage area ratios.

The configuration charts shown in Figures 2-1 to Figure 2-6 (Pages 12 to 17) show where ungauged local inflows are incorporated into the natural flow model.

TABLE 2.6-1

ESTIMATION OF UNGAUGED LOCAL INFLOW

Ungauged Location	GDA (km ²)	EDA (km ²)	Transfer Station	GDA Ratio	EDA Ratio	SSARR Station
Above Eyebrow L.	1086	176	Ridge Cr. Nr. Bridgeford 05JG013	2.36	1.07	31101
Above Buffalo Pound	688	340	Q.R. Ab. Buffalo Pound (Natural 1600)	0.26	0.39	31700
MJR Nr. Burdick to BP	215	136	MJR Nr. Burdick 05JE006	0.02	0.04	32501
Q.R. below Moose Jaw R. to Q.R. Nr. Lumsden	880	403	Boggy Cr. Nr. Lumsden 05JF006	2.19	1.29	34122
Q.R. Nr. Lumsden to Q.R. below Flying Cr.	746	605	Boggy Cr. Nr. Lumsden 05JF006	0.86	0.95	35350
Q.R. below Loon Cr. to Echo Lake Outlet	3047	940	Indianhead Cr. Nr. Indian Head 05JL002	9.32	5.00	37199
Q.R. below Echo L. to Katepwa L. Outlet	982	435	Indianhead Cr. Nr. Indian Head 05JL002	3.00	2.31	37399
Q.R. below Katepwa L. to Q.R. at Hyde	3239	1496	Indianhead Cr. Nr. Indian Head 05JL002	9.91	7.96	37565
Q.R. at Hyde to Crooked L. Outlet	1002	420	Ekapo Cr. Nr. Marieval 05JM010	0.91	0.95	37699
Q.R. below Katepwa L. to Round L. Outlet	1452	724	Ekapo Cr. Nr. Marieval 05JM010	0.32	0.64	37799
Q.R. below Round L. to Q.R. near Welby	4224	1814	Cutarm Cr. Nr. Spyhill 05JM015	4.49	3.56	37899

Note: GDA - gross drainage area
EDA - effective drainage area

Source of Drainage Areas - PFRA Hydrology Report #104, January 1988

The estimated return period of the year being simulated is specified using 6S records. The model calculates the ungauged local inflow using the recorded flow of the specified control station, the specified return period and the calculated transfer ratio specified in three-dimensional tables (6F records).

The model uses the effective drainage area ratio to transfer flows in runoff events less than or equal to a 1:2 runoff event. The gross drainage area ratio is used to transfer flows in runoff events larger than or equal to a 1:500 runoff event. The model linearly interpolates between the EDA ratio and the GDA ratio for events between the 1:2 and the 1:500 runoff events. The above relationships are incorporated into the model using three-dimensional tables (CF records).

With these local inflows included, the flows and elevations simulated by the existing conditions limb of the model can now be better compared with the recorded data. In this way, the accuracy of the model simulation can be determined.

Reach outflows simulated in the existing conditions limb of the model are subtracted from the respective recorded flows. The difference in flow is a combination of groundwater inflow or outflow and a correction of the estimated ungauged inflow. This groundwater and ungauged inflow adjustment is then added to the natural flow limb. In all reaches, with the exception of the Loon Creek to below Katepwa Lake reach, the groundwater inflow and ungauged inflow adjustment is added to the outflow simulated in the

corresponding natural flow limb. In the Loon Creek to below Katepwa Lake reach, the groundwater inflow and ungauged inflow adjustment is added to the natural flow limb, half above Echo Lake and half above Katepwa Lake. In this way, the adjustment is added directly to the actual location where the groundwater inflow and ungauged inflow most likely occur.

2.7 Groundwater Inflow

Groundwater is an important component of the Qu'Appelle River hydrologic regime. The report prepared by the Qu'Appelle Basin Study Board estimated the Qu'Appelle River system receives an estimated 40000 acre-feet (49340 dam³) of water annually from groundwater (Report of the Qu'Appelle Basin Study Board, Canada - Saskatchewan - Manitoba, 1972). The major source of groundwater discharge into the Qu'Appelle River Valley is the Hatfield Valley Aquifer (Fort Qu'Appelle Geology, Saskatchewan Research Council, 1977).

In the spring runoff portion of the original Qu'Appelle River Natural Flow Model (Q12), the effect of groundwater is accounted for in the net inflow factor calculated for Last Mountain Lake and in the groundwater and ungauged inflow adjustment factor in the Fishing Lakes. There is not, however, any compensation for groundwater inflow into either Last Mountain Lake or the Fishing Lakes in the fall and winter portion of the original natural flow model. The original fall and winter model simply started the lakes at the August 1 elevation, simulated by the spring and summer model, and routed them through the fall and winter months. The original model assumed no tributary inflow and no groundwater inflow in the fall and winter period.

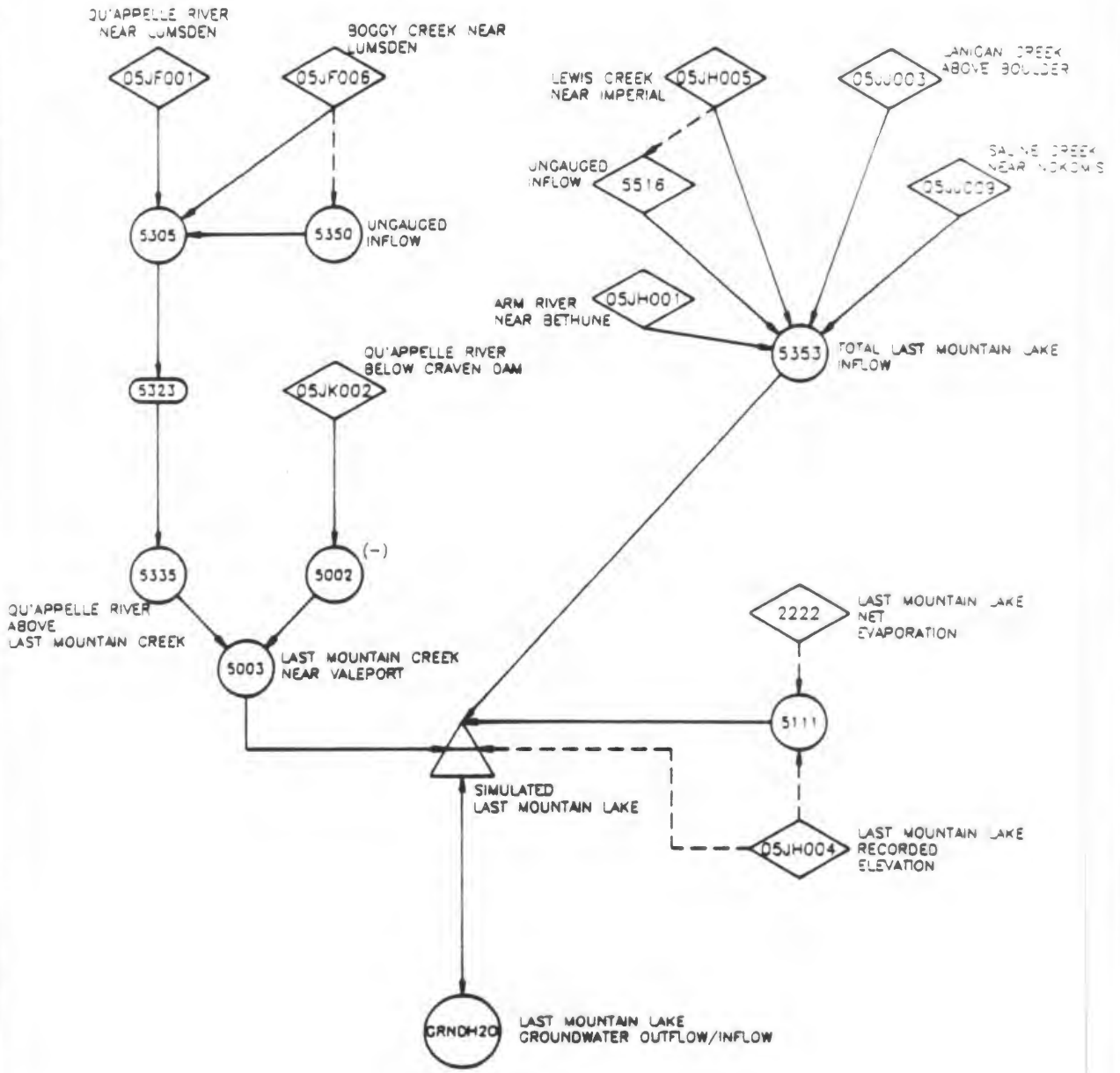
Water balances were carried out on Last Mountain Lake and the Fishing Lakes to determine the amount of groundwater inflow into these lakes during the fall and winter period.

2.7.1 Last Mountain Lake

Two separate water balances were carried out on Last Mountain Lake to determine the amount of groundwater inflow into Last Mountain Lake. The first method consisted of completing a daily water balance on Last Mountain Lake for the Period of 1975 to 1986. The second method of analysis consisted of completing a monthly water balance for the months of December and January over the period of 1975 to 1988. The following two sections discuss each water balance method in detail.

2.7.1.1 Method I - Daily Water Balance

The SSARR model was used to simulate groundwater inflow into Last Mountain Lake on a daily basis for the period of 1975 to 1986. The configuration chart for the model which was developed for this purpose is shown in Figure 2.7.1.1-1. As shown in this figure, recorded tributary flows were input into Last Mountain Lake; ungauged inflow was estimated using Lewis Creek near Imperial. Evaporation from Last



LAST MOUNTAIN LAKE
GROUND WATER SSARR MODEL
CONFIGURATION CHART

Figure 2.7.1.1-1

Mountain Lake was estimated using the recorded lake elevation and mean monthly net evaporation estimated for the Last Mountain Lake area; the net flow in Last Mountain Creek was estimated by subtracting the recorded flow below Craven Dam from the estimated flow in the Qu'Appelle River above Last Mountain Creek. The simulated elevation of Last Mountain Lake was forced to equal the actual recorded elevation. The SSARR model then calculated the daily lake outflow required to satisfy the water balance. This calculated daily outflow is attributed to groundwater flow.

The daily groundwater inflow rates calculated with this model were very erratic. Last Mountain Lake has a large surface area, thus even small changes in the recorded daily lake elevation indicate a relatively large change in the volume of water stored in the lake. As a result, even small inaccuracies in the recorded lake elevation data due to wind set up, recording gauge anomalies, and even rounding errors, have a significant effect on the water balance results.

Although the daily groundwater flow rates were erratic, the annual volumes of groundwater inflow into Last Mountain Lake, over the period of 1975 to 1986, that were simulated using this method were fairly consistent as shown in Figure 2.7.1.1-2. The mean annual volume of groundwater inflow into Last Mountain Lake was 41800 acre-feet/year (51560 dam³).

LAST MOUNTAIN LAKE

SIMULATED ANNUAL GROUNDWATER INFLOW VOLUME
1975-1986

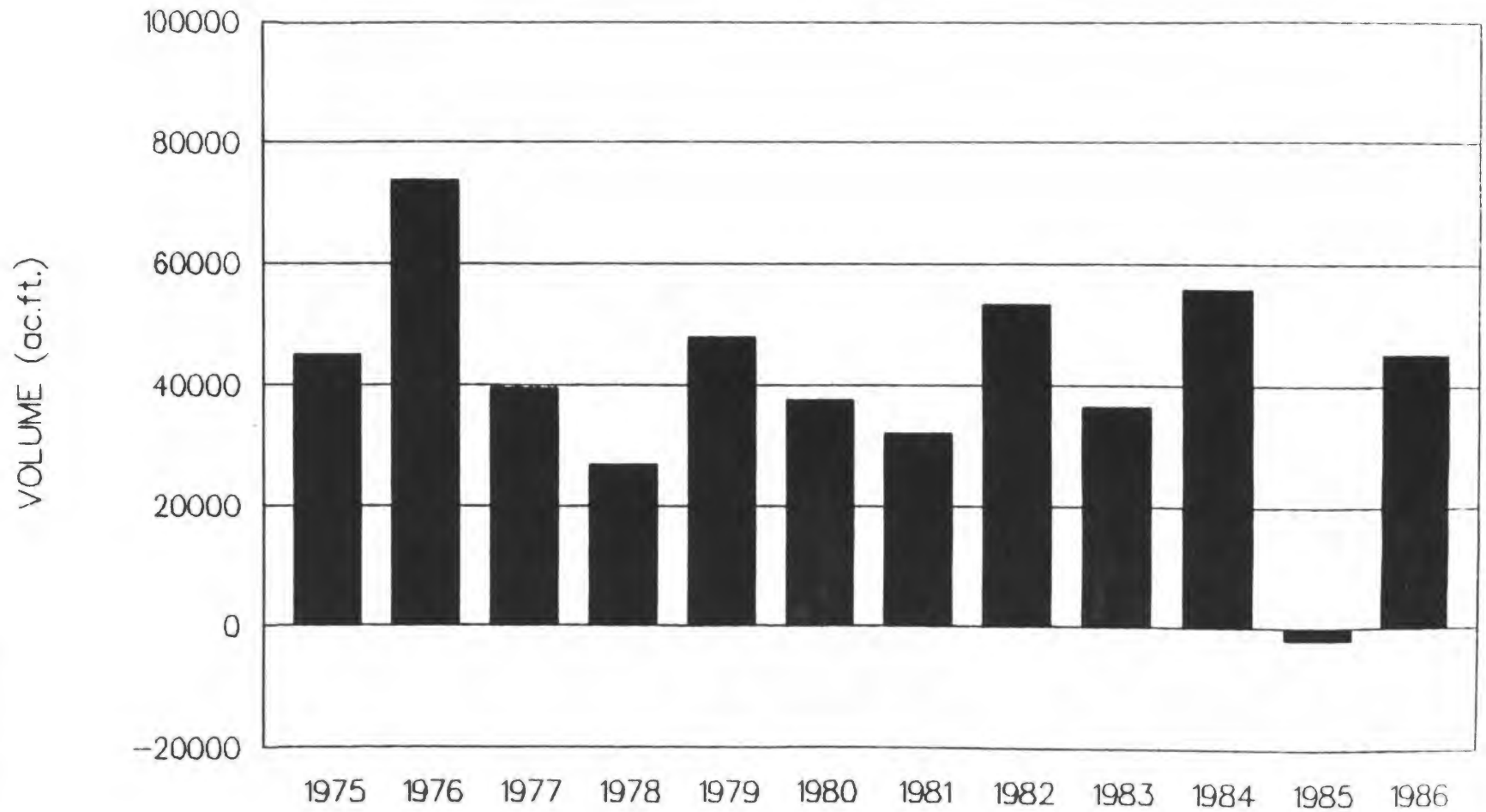


Figure 2.7.1.1-2

LAST MOUNTAIN LAKE

AVERAGE DAILY GROUNDWATER FLOW (1975 TO 1986)
30 DAY MOVING MEAN



Figure 2.7.1.1-3

The average daily groundwater inflow rate for the 1975 to 1986 period is plotted in Figure 2.7.1.1-3. This figure shows that the 41800 acre-feet of groundwater inflow is not distributed evenly throughout the year. However, this figure does show the groundwater flow rate staying relatively constant over the winter period. The mean daily groundwater inflow rate into Last Mountain Lake for the months of December and January was 31.1 cfs ($0.88 \text{ m}^3/\text{s}$). Figure 27.1.1-3 shows a dramatic increase in groundwater inflow during the summer months. It is suggested that this dramatic increase could be due in part to an under-estimation of local inflow and/or an over-estimation of evaporation losses. However, this anomaly in the water balance during the summer months is not a concern as we are interested in obtaining a groundwater inflow estimate in the winter months only.

2.7.1.2 Method II - Monthly Water Balance

The second method of estimating groundwater inflow into Last Mountain Lake consisted of completing water balances on Last Mountain Lake on a monthly basis. Since groundwater inflow is accounted for in the net inflow factor in the spring and summer portion of the model, only the winter months of December and January were considered in this analysis.

LAST MOUNTAIN LAKE

MEAN DECEMBER AND JANUARY GROUNDWATER INFLOW
(DECEMBER 1975 TO JANUARY 1988)

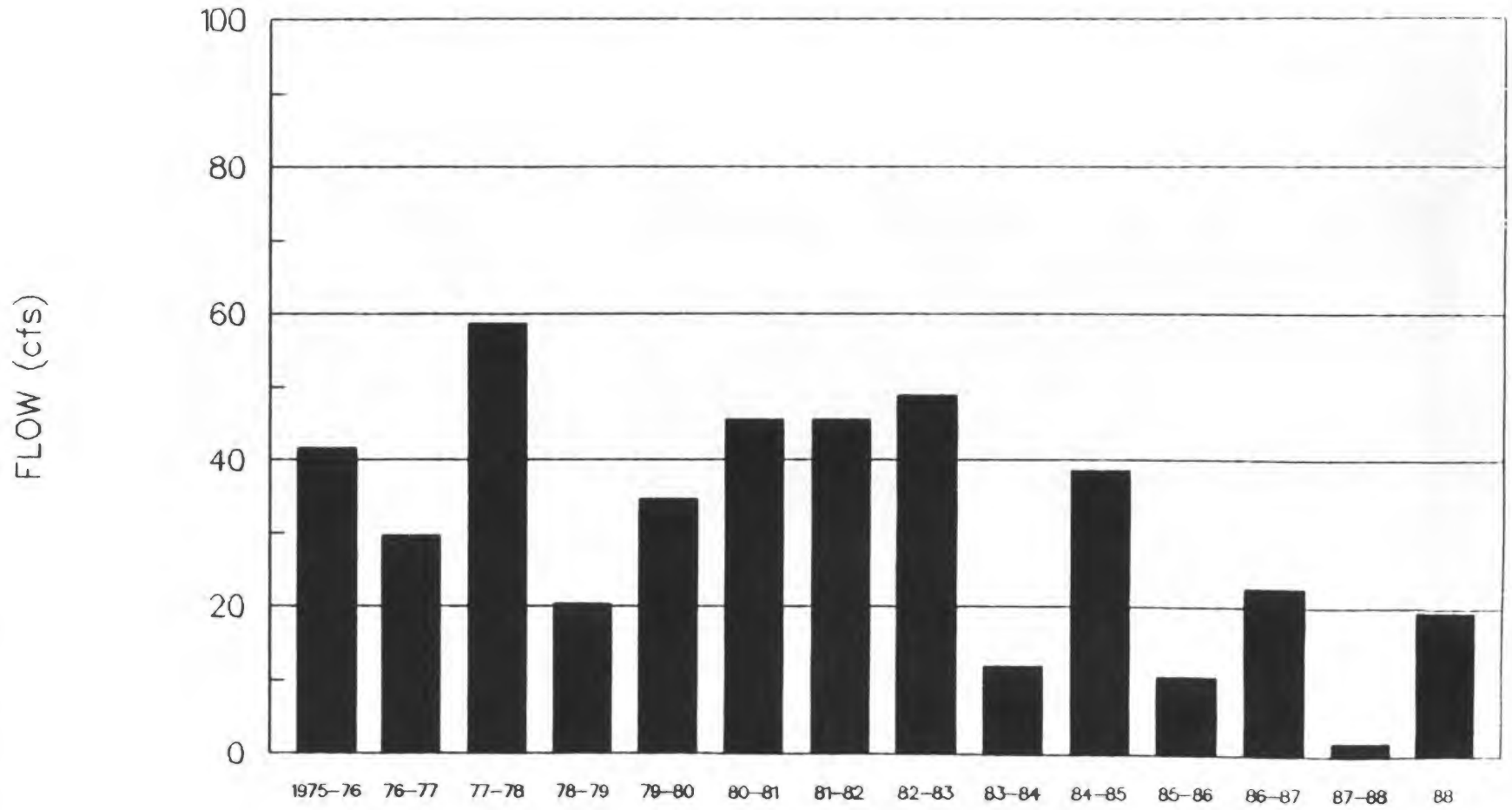


Figure 2.7.1.1-2-1

Using these two winter months simplified the water balance calculation because in these months both net evaporation and local inflow could be assumed to be zero. The simplified water balance, equation (2), could easily be solved to determine the monthly volume of groundwater inflow.

$$\text{Groundwater Inflow} = (\text{Q.R. below Flying Creek} - \text{Q. R. below Craven}) + \text{Change in Last Mountain Storage.} \quad (2)$$

The mean daily rate of groundwater inflow, calculated for the months of December and January for the period of 1975 to 1988 are plotted in Figure 2.7.1.2-1. The mean daily groundwater inflow rate for the months of December and January calculated using this method was 30.8 cfs ($0.87 \text{ m}^3/\text{s}$).

The average mean daily winter groundwater inflow into Last Mountain estimated using the two methods was 31 cfs ($0.88 \text{ m}^3/\text{s}$). The configuration chart shown in Figure 2-7 (Page 18) shows how the 31 cfs groundwater inflow into Last Mountain Lake was incorporated into the winter portion of the modified natural flow model.

2.7.2 Fishing Lakes

In order to determine the effect of groundwater on the Fishing Lakes, a monthly water balance was completed. Since groundwater inflow into

the Fishing Lakes is already accounted for in the spring and summer period, only winter months were considered in this analysis.

Using only the months of December and January simplified the water balance equation as both net evaporation and tributary inflow could be assumed to be negligible. Thus, the simplified water balance, equation (3), could easily be solved.

$$\text{Groundwater Inflow} = \text{Change in Lake Storage} - (\text{Q.R. below Loon Creek} - \text{Q.R. below Katepwa Lake}) \quad (3)$$

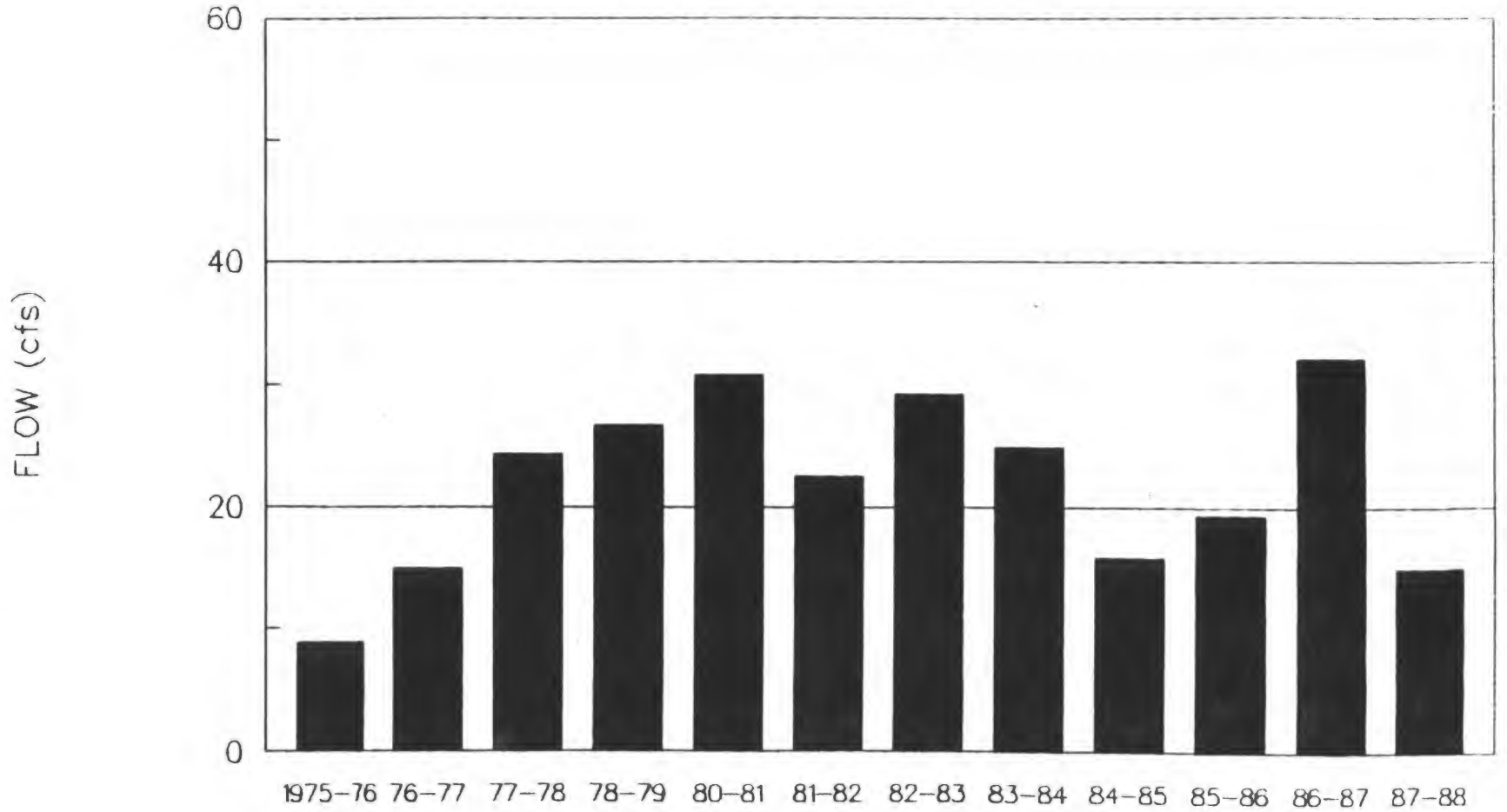
Results of the water balance for the period of 1975 to 1988 are shown in Figure 2.7.2-1. This figure shows the calculated December and January groundwater inflow relatively constant for the 1975 to 1988 period. The mean groundwater inflow rate calculated was 22 cfs (0.62 m³/s). The model configuration chart shown in Figure 2-8 shows how the groundwater inflow into the Fishing Lakes was incorporated into the winter portion of the model. The 22 cfs groundwater inflow was split equally between Echo Lake and Katepwa Lake.

2.8 Other Modifications

During the process of completing the modifications outlined in the Memorandum of Understanding, a number of additional modifications were made to the Qu'Appelle River Natural Flow Model. These additional modifications are discussed below.

FISHING LAKES

MEAN DECEMBER AND JANUARY GROUNDWATER INFLOW
(DECEMBER 1975 TO JANUARY 1988)



NOTE : DECEMBER 1979 DATA NOT AVAILABLE

2.8.1 Summation Reservoirs

In order to make the model easier to use by reducing the number of manual calculations required, summation reservoirs were placed at a number of locations where the total volume of flow is required. Table 2.8-1 lists the locations where summation reservoirs were incorporated into the model. To avoid confusion by further complicating the model configuration charts, these summation reservoirs were not shown in Figures 2-1 to 2-10 (Pages 12 to 20).

These summation reservoirs were initialized at the beginning of each run to have zero storage. The elevation/outflow relationship for these reservoirs were set such that there is no outflow at all reservoir elevations. Thus, the volume of water stored in these reservoirs at the end of a period is equal to the total volume of flow simulated for the respective flow station. Monthly volumes of flow can be determined by merely subtracting month-end storage in the reservoir.

These summation reservoirs revealed two apparent anomalies in the SSARR program. First, flows decrease to a value of -1, not zero as would be expected. Secondly, the increase in the volume of storage in the summation reservoirs does not correspond to the inflow unless lcfs is added to the flow.

TABLE 2.8-1

SUMMATION RESERVOIRS

Summation Reservoir Location	SSARR Station No.
Elbow Diversion	S1000
Qu'Appelle River Below Moose Jaw River - Recorded	S4000
Qu'Appelle River Below Moose Jaw River - Natural	S94000
Qu'Appelle River Near Lumsden - Recorded	S5000
- Natural	S94900
Qu'Appelle River Below Craven Dam - Recorded	S6000
Qu'Appelle River Below Craven Dam - Natural	S95999
Cumulative Evaporation - Natural Lakes	SEVPN
Cumulative Evaporation - Recorded Lakes	SEVPS
Qu'Appelle River Below Katepwa Lake - Recorded	S7500
- Simulated	S7499
- Natural	S97500
Qu'Appelle River Near Welby - Recorded	S8000
- Simulated	S7984
- Natural	S97999

2.8.2 Data Records

In analyzing the original model input files used to estimate the natural flow in the Qu'Appelle River for the years 1975 to 1988, numerous errors in the input data records were found. It is suspected that the major cause of these errors was due to a misunderstanding of the configuration of these data input records.

Man-made releases and storages and other data were often entered with 6D data records with only one data point per record, with the understanding that the model would interpolate between the specified points. The SSARR model does interpolate between specified values, however, if the number of data points on the record is not specified as was often the case, the model defaults to assume the record contains the full number data points (8 points). Thus, instead of interpolating between two intended data points, the model reads seven data points of zero flow after the first specified flow. The model then interpolates between the last zero read and the next specified data point.

When re-running the model over the 1975 to 1988 period, much of the data which must be entered manually (man-made releases, storages, LML net inflow) was taken from the original files, however, care was taken to ensure the data records were in the correct format.

2.9 Re-Running the Modified Model

2.9.1 Starting Year

In order to determine the effect of the modifications made to the Qu'Appelle River Natural Flow Model, natural flows and elevations simulated with the modified model for 1986 could not simply be compared with those simulated with the original model. Because final simulated natural lake elevations carry over as the starting simulated natural lake elevations for the following simulation run, the effects of the modifications are cumulative. Thus, it was necessary to re-run the model for a number of years, starting with the year with the last known natural lake elevation.

In 1974, a combination of precipitation accumulation and snowmelt runoff produced the flood of record in the upper Qu'Appelle River Basin. At most flow gauging stations throughout the basin, the highest annual volumes on record were recorded in 1974. Throughout the flood of 1974, control structures were left wide open in an attempt to minimize the amount of damage caused by flooding. With all control structures completely open, the effects of man-made changes to the basin were minimal, and the runoff proceeded under what can be considered as near natural conditions. With the exception of the Buffalo Pound Outlet control, all control structures remained wide open until the spring of 1975. Thus, it could be assumed that on March 1, 1975 the elevations of the lakes throughout the Qu'Appelle River Basin under natural conditions would approximately be equal to the recorded lake elevations on that date.

Buffalo Pound Lake's natural elevation was assumed to be at the natural outlet elevation on March 1, 1975.

1976 was also a high runoff year in the Qu'Appelle River Basin, and similar to 1974, all control structures were left completely open for the entire year. However, it was decided to re-run the model starting in the spring of 1975 so that the performance of the modified model in a high runoff year (1976) would be tested.

2.9.2 Modified Model Input Data Requirements

As a result of the numerous modifications made to the configuration of the original Qu'Appelle River Natural Flow Model, the data set-up requirements of the modified model are slightly different from those of the original model. Tables 2.9.2-1 and 2.9.2-2 itemize the changes in the input data requirements for the spring and summer, and fall and winter portions of the model respectively. Table 2.9.2-1 also lists the source of the additional data. The format in which these data are to be entered into the modified model are shown in the example input files found in Appendix D. When re-running the model over the 1975 to 1988 period, gross evaporation estimated by PFRA was used (Hydrology Report #121) for the entire period. When running the model for apportionment purposes in the future, average gross evaporation, published by the PFRA hydrology division should be used for the April to June apportionment report. Monthly Gross evaporation Data, estimated using Meyer's equation, should be used for the annual apportionment report.

TABLE 2.9.2-1

CHANGES IN INPUT DATA REQUIREMENTS
SPRING AND SUMMER MODEL (Q12)

SSARR Station No.	Description	Change	Source
7035	Man-Made Storage - Fishing Lakes	No Longer Required	----
97756	Man-Made Storage - Crooked Lake	No Longer Required	----
97806	Man-Made Storage - Round Lake	No Longer Required	----
27199	Outlet Structure Elevation - Echo Lake	Must be Specified with 6S Cards	PFRA, Operations Reports
27699	Outlet Structure Elevation - Crooked Lake	Must be Specified with 6S Cards	PFRA, Operations Reports
27799	Outlet Structure Elevation - Round Lake	Must be Specified with 6S Cards	PFRA, Operations Reports
47199	Riparian Release - Echo Lake	Must be Specified with 6S Cards	Sask Water
47399	Riparian Release - Katepwa Lake	Must be Specified with 6S Cards	Sask Water
7985	Cutarm Creek near Spy Hill - 05JM015 Recorded Daily Streamflow	Streamflow Data Required in 6D Form	Environment Canada, Water Resources Branch
RETPER	Estimated Return Period of Year Being Simulated	Must be Specified with 6S Cards	Sask Water
1111	Net Evaporation - Buffalo Pound	Must be Specified with 6S Cards	Environment Canada, Atmospheric Environment Service
2222	Net Evaporation - Last Mountain Lake	Must be Specified with 6S Cards	Environment Canada, Atmospheric Environment Service
7176	Net Evaporation - Fishing Lakes	Must be Specified with 6S Cards	Environment Canada, Atmospheric Environment Service
7668	Net Evaporation - Crooked and Round Lakes	Must be Specified with 6S Cards	Environment Canada, Atmospheric Environment Service
1790	Recorded Daily Elevation - Buffalo Pound	Must be Specified with 6D Cards	Environment Canada, Water Resources Branch
86000	Recorded Daily Elevation - Last Mountain	Must be Specified with 6D Cards	Environment Canada, Water Resources Branch
7200	Recorded Daily Elevation - Echo Lake	Must be Specified with 6D Cards	Environment Canada, Water Resources Branch
7350	Recorded Daily Elevation - Katepwa Lake	Must be Specified with 6D Cards	Environment Canada, Water Resources Branch
7600	Recorded Daily Elevation - Crooked Lake	Must be Specified with 6D Cards	Environment Canada, Water Resources Branch
7700	Recorded Daily Elevation - Round Lake	Must be Specified with 6D Cards	Environment Canada, Water Resources Branch

TABLE 2.9.2-2

CHANGES IN INPUT DATA REQUIREMENTS
FALL AND WINTER MODEL (Q3)

SSARR Station No.	Description	Change
1791	Buffalo Pound Aug. 1 Natural Lake Elevation	Specify Aug.1 Natural Lake Elevation with 2L Card
95549	Last Mountain Lake Aug. 1 Natural Lake Elevation	Specify Aug.1 Natural Lake Elevation with 2L Card
97197	Echo Lake Aug. 1 Natural Lake Elevation	Specify Aug.1 Natural Lake Elevation with 2L Card
97397	Katepwa Lake Aug. 1 Natural Lake Elevation	Specify Aug.1 Natural Lake Elevation with 2L Card
97697	Crooked Lake Aug. 1 Natural Lake Elevation	Specify Aug.1 Natural Lake Elevation with 2L Card
97797	Round Lake Aug. 1 Natural Lake Elevation	Specify Aug.1 Natural Lake Elevation with 2L Card
1111	Buffalo Pound Net Evaporation	Must be Specified with 6S Cards
2222	Last Mountain Lake Net Evaporation	Must be Specified with 6S Cards
7176	Fishing Lakes Net Evaporation	Must be Specified with 6S Cards
7668	Crooked & Round Lakes Net Evaporation	Must be Specified with 6S Cards

2.9.3 Procedure for Running Modified Model

Switching the model from the mainframe computer to the microcomputer version has altered the procedure for running the model. The new procedure, shown in Appendix C, was obtained by capturing messages to the microcomputer screen on a printer during a typical model running session.

Sample input files for the spring and summer, and for the fall and winter portion of the modified model are shown in Appendix D. Also found in Appendix D are sample input files which were used to specify which stations were to be included in the model outputs. Example output files obtained using the modified spring and summer and fall and winter models can be found in Appendix E.

CHAPTER III
DISCUSSION OF RESULTS

3.1 General Performance of Modified Model

When re-running the model over the 1975 to 1988 period, the simulated results for each year, for a number of key locations throughout the Qu'Appelle River Basin, were plotted to ensure the modified model was working properly. Figures 3.1-1 to 3.1-10 show the simulation results produced by the spring and summer portion of the modified model for a median runoff year (1986). These figures show simulated natural, simulated existing conditions and recorded elevations and flows for 10 key locations throughout the Qu'Appelle River Valley. These figures show the modified model simulating reasonable lake levels and streamflows for both existing and natural conditions. Similar results were obtained for all 14 years (1975 to 1988) which were simulated.

Lake elevations simulated by the existing conditions limb of the modified model are shown in Figures 3.1-5, 3.1-6, 3.1-8 and 3.1-9. These figures show the modified model simulated lake elevations which are close to actual recorded elevations. However, the simulated August 1 elevations are not exactly equal to the recorded August 1 lake elevations. The difference between the August 1 volume of water in lake storage under recorded conditions and the volume of water in lake storage under simulated conditions must be accounted for as this is water which should have been routed through the system. The volume of water in lake storage on August 1

BUFFALO POUND LAKE ELEVATION

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

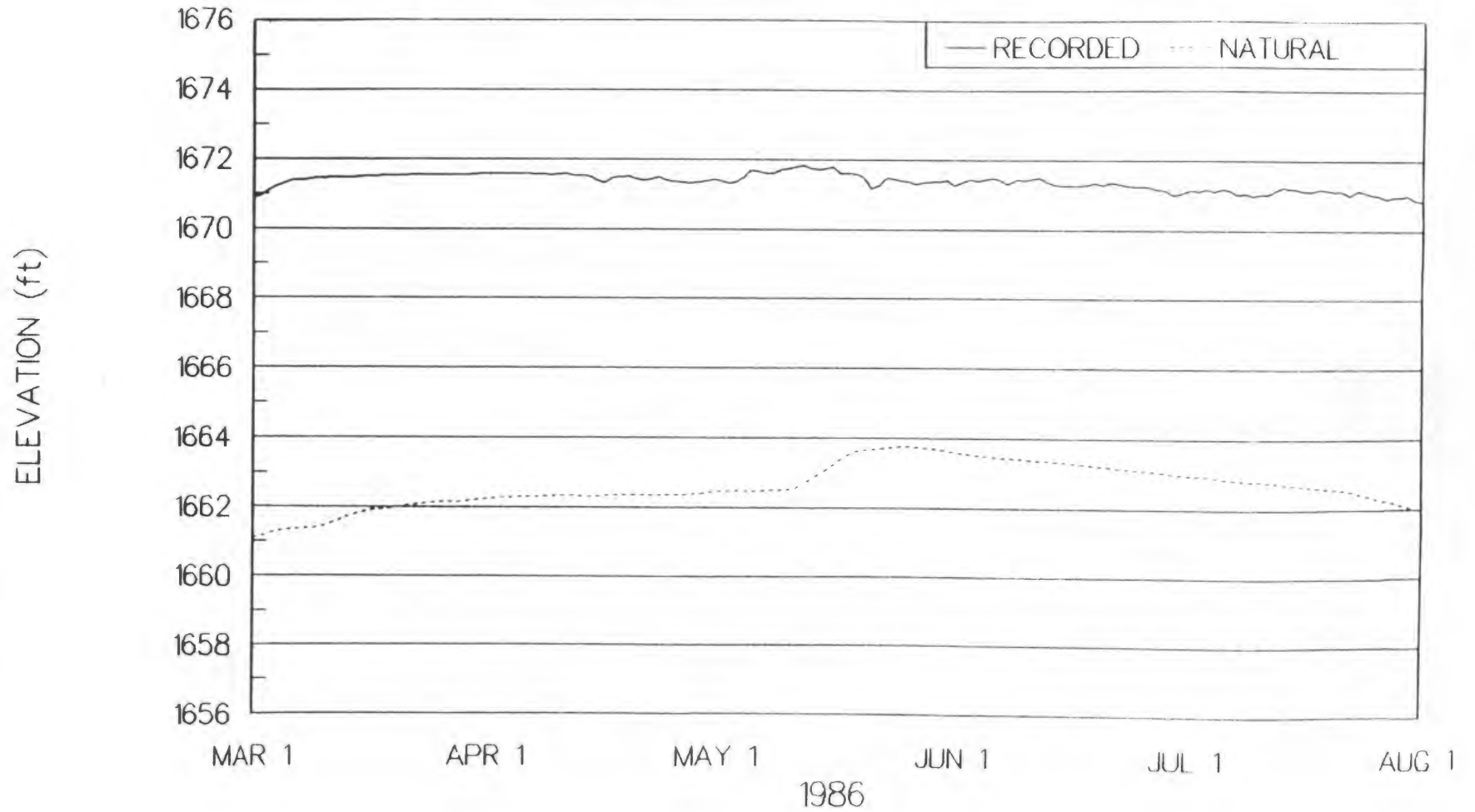


Figure 3.1-1

QU'APPELLE RIVER BELOW MOOSE JAW RIVER

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

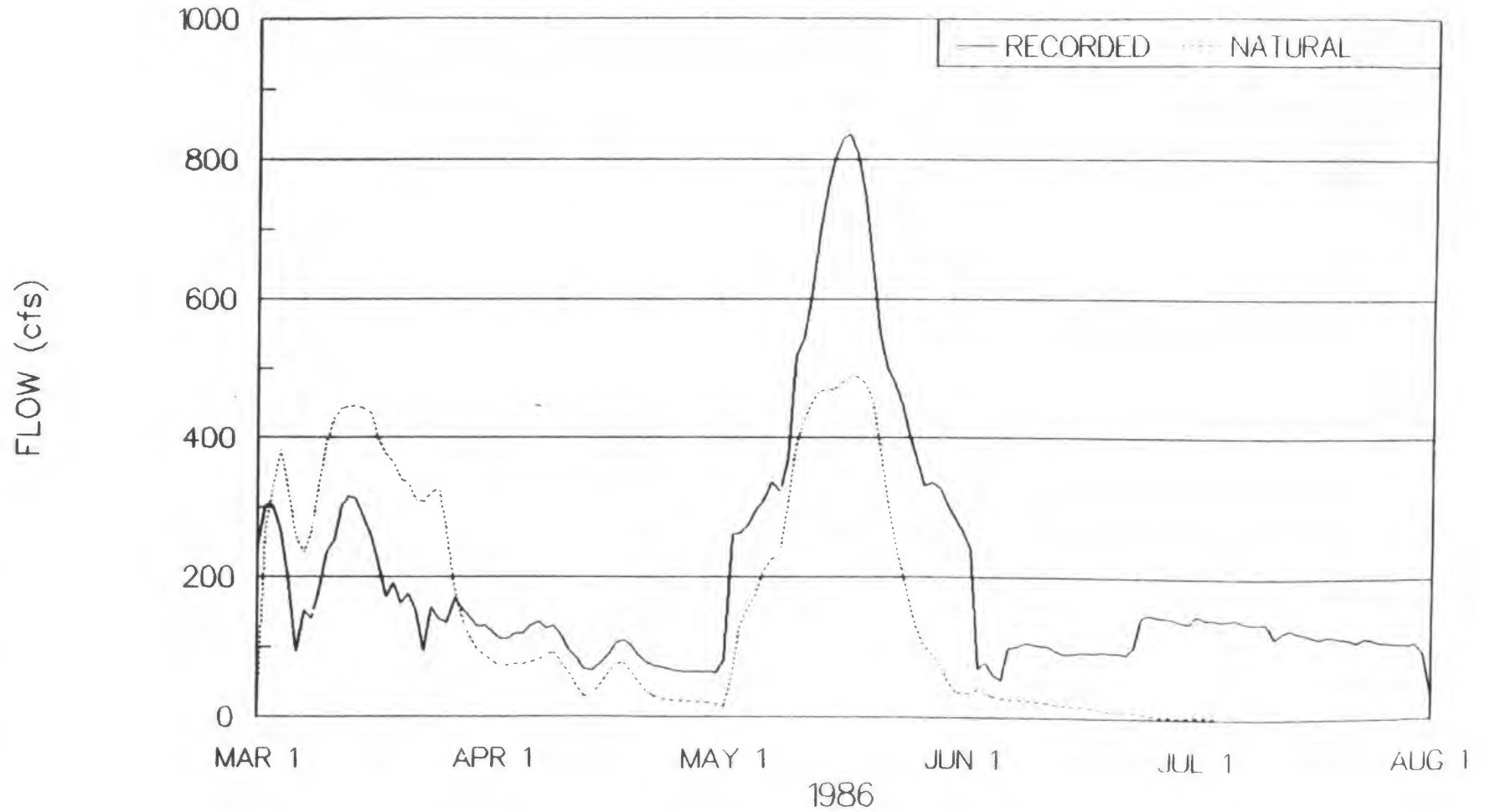


Figure 3.1-1-2

LAST MOUNTAIN LAKE ELEVATION

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

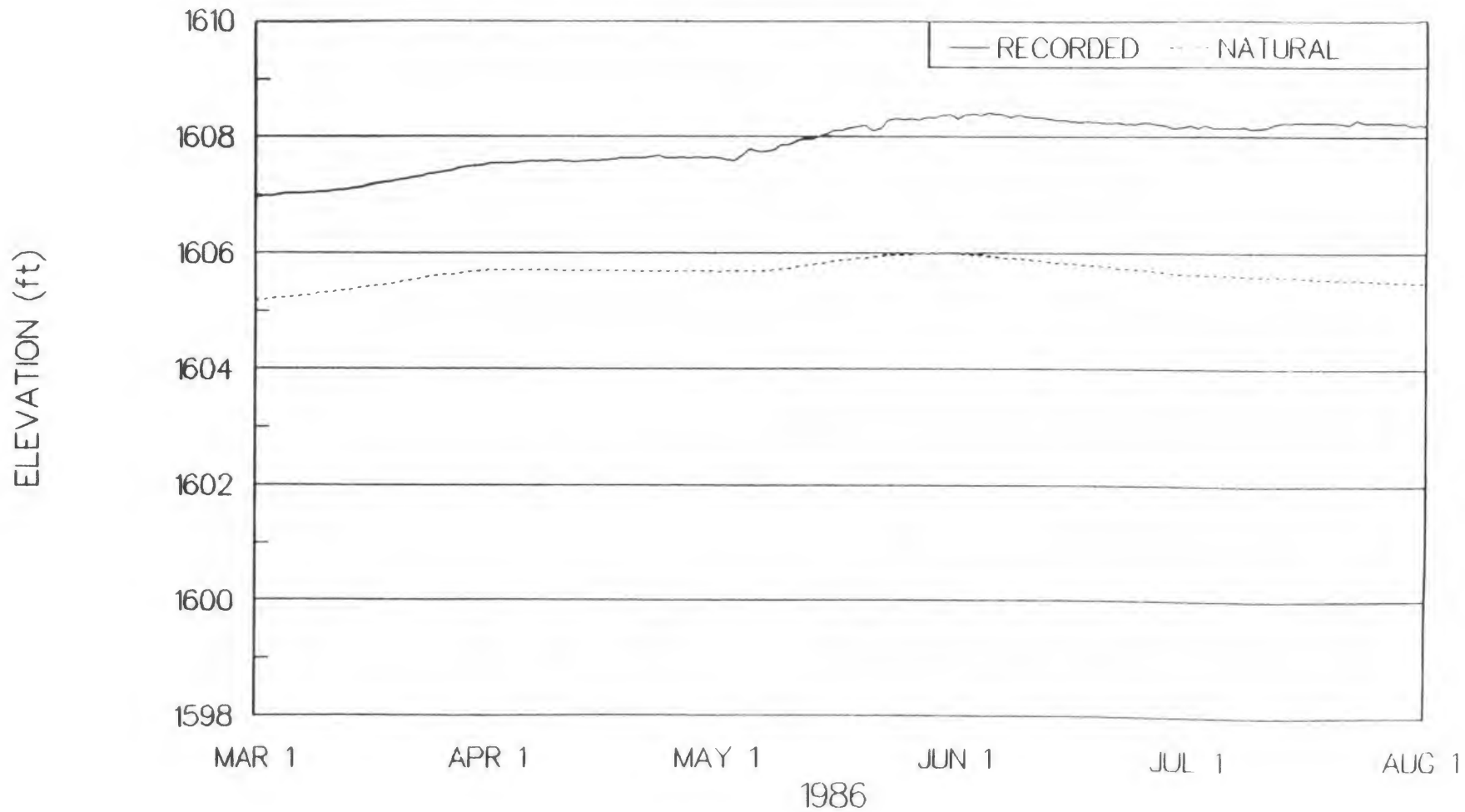


Figure 3.1-3

QU'APPELLE RIVER BELOW CRAVEN DAM

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

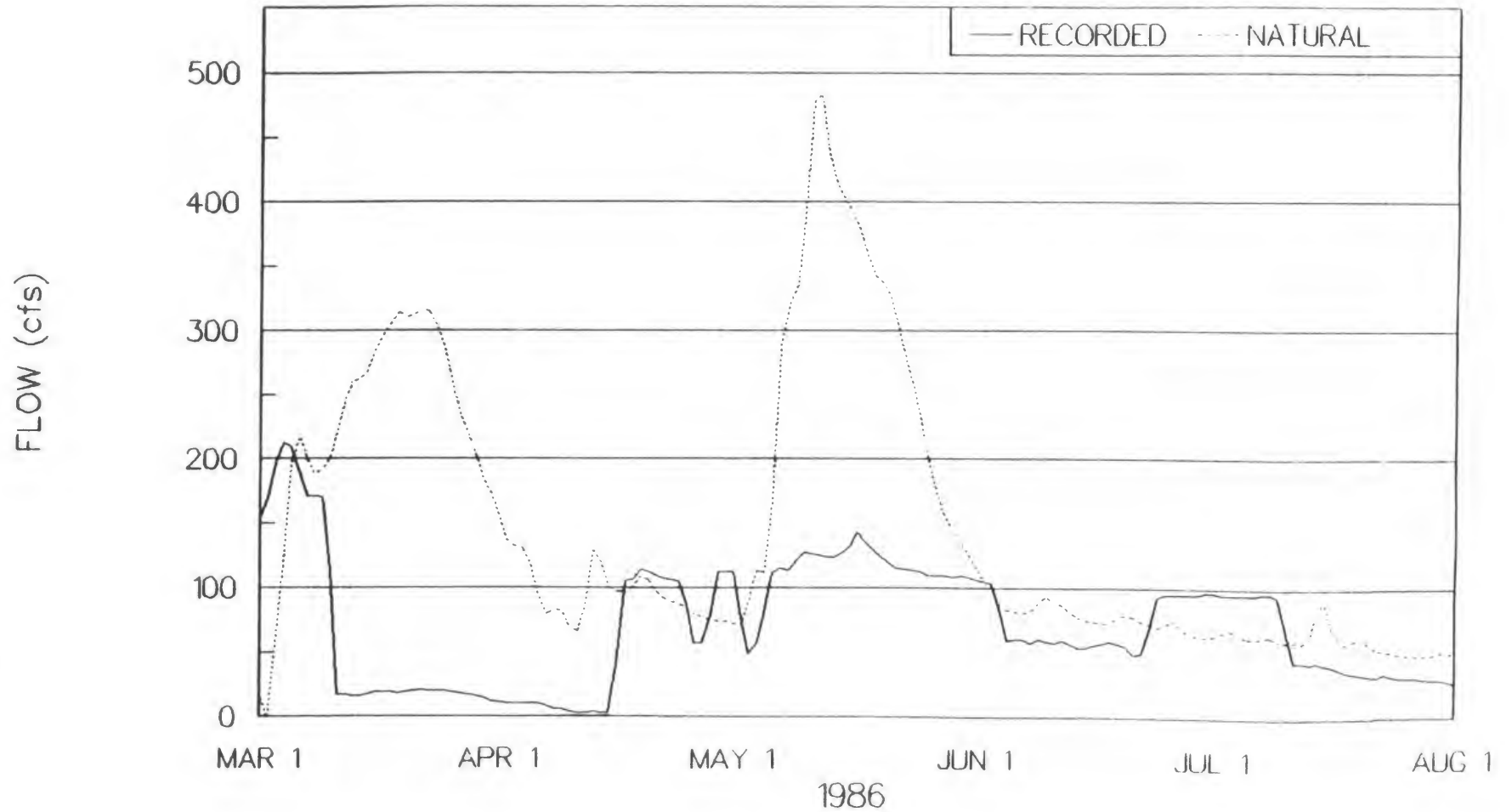


Figure 3.1-4

ECHO LAKE ELEVATION

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

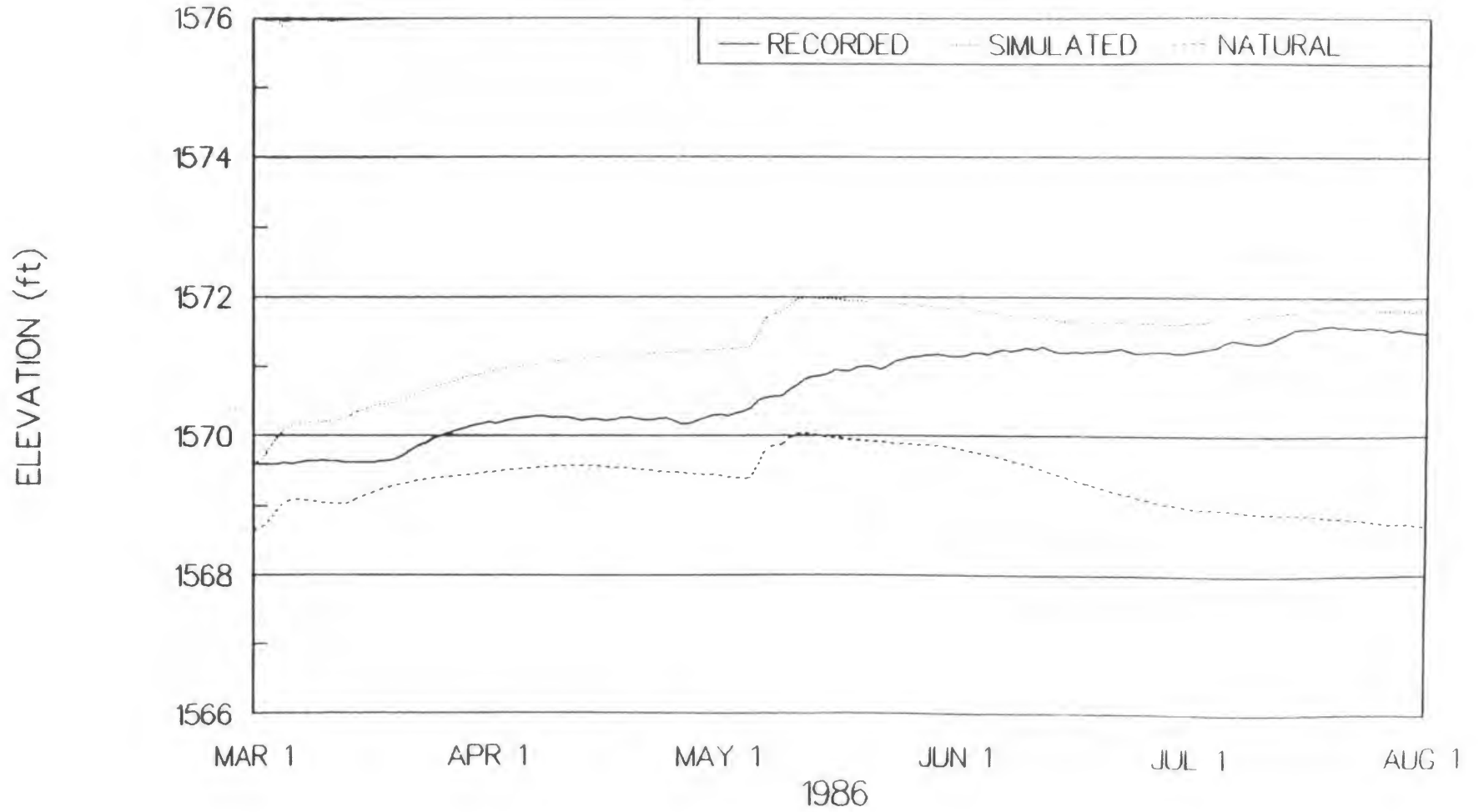


Figure 3.1-5

KATEPWA LAKE ELEVATION

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

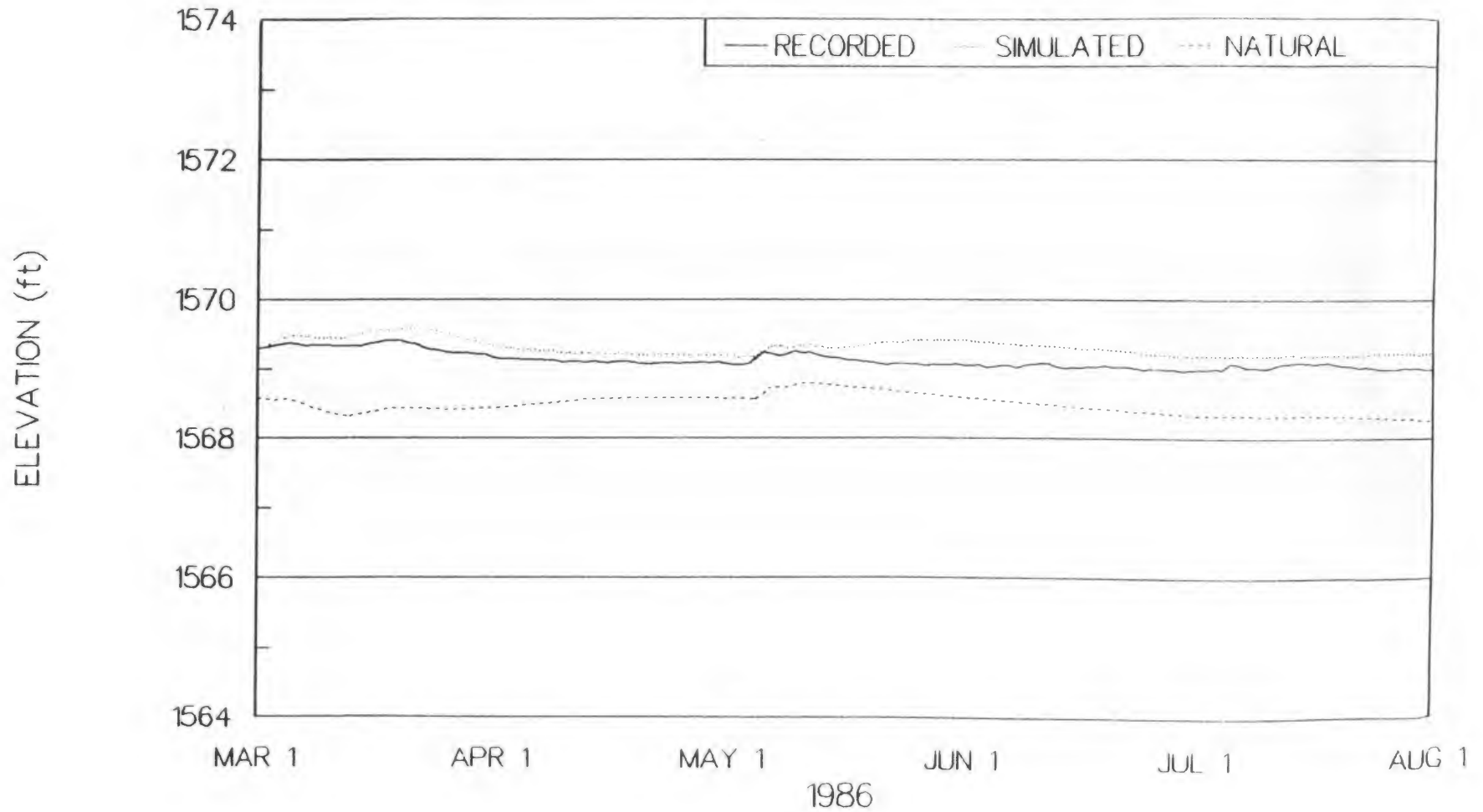


Figure 3.1-6

QU'APPELLE RIVER BELOW KATEPWA LAKE

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

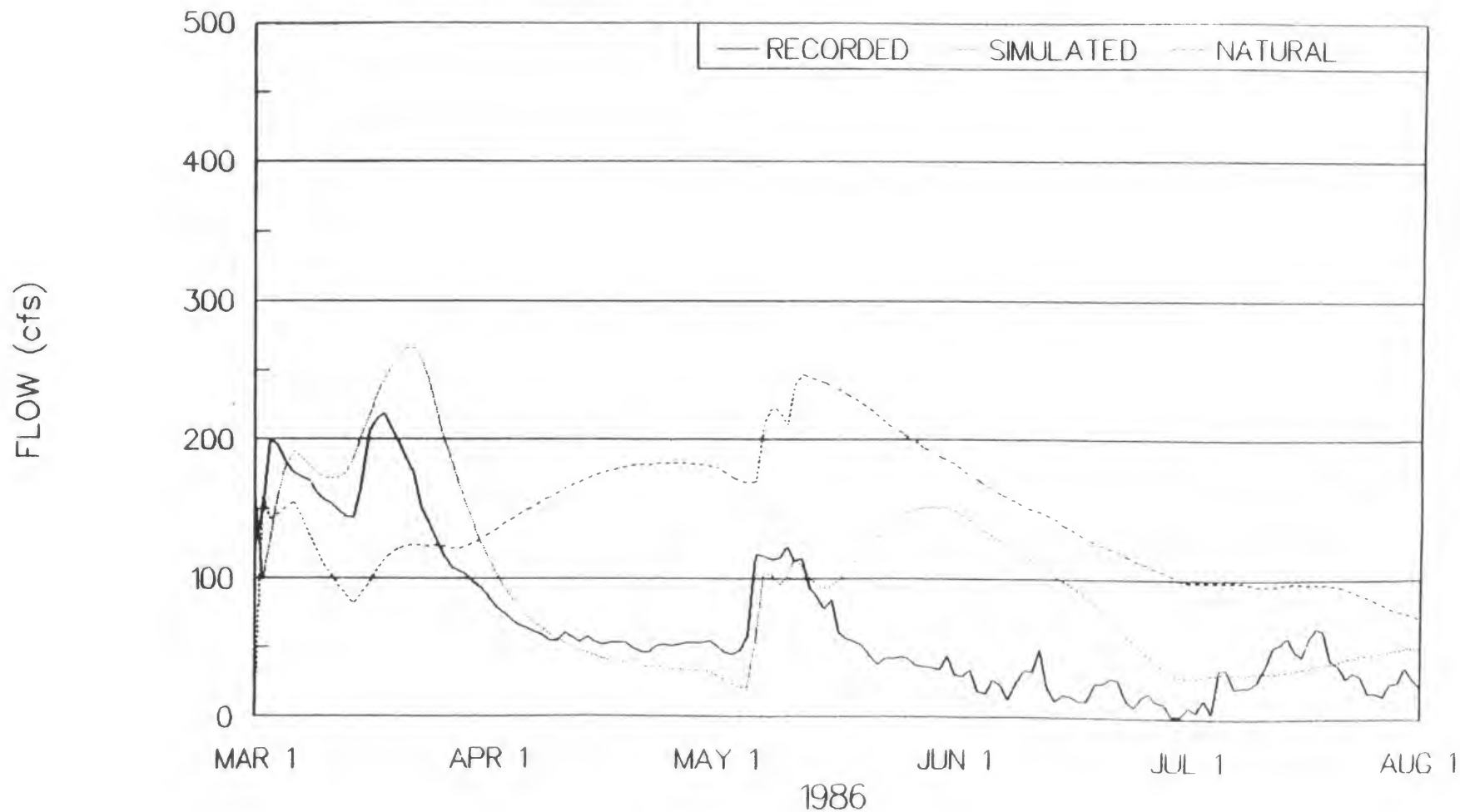


Figure 3.1-7

CROOKED LAKE ELEVATION

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

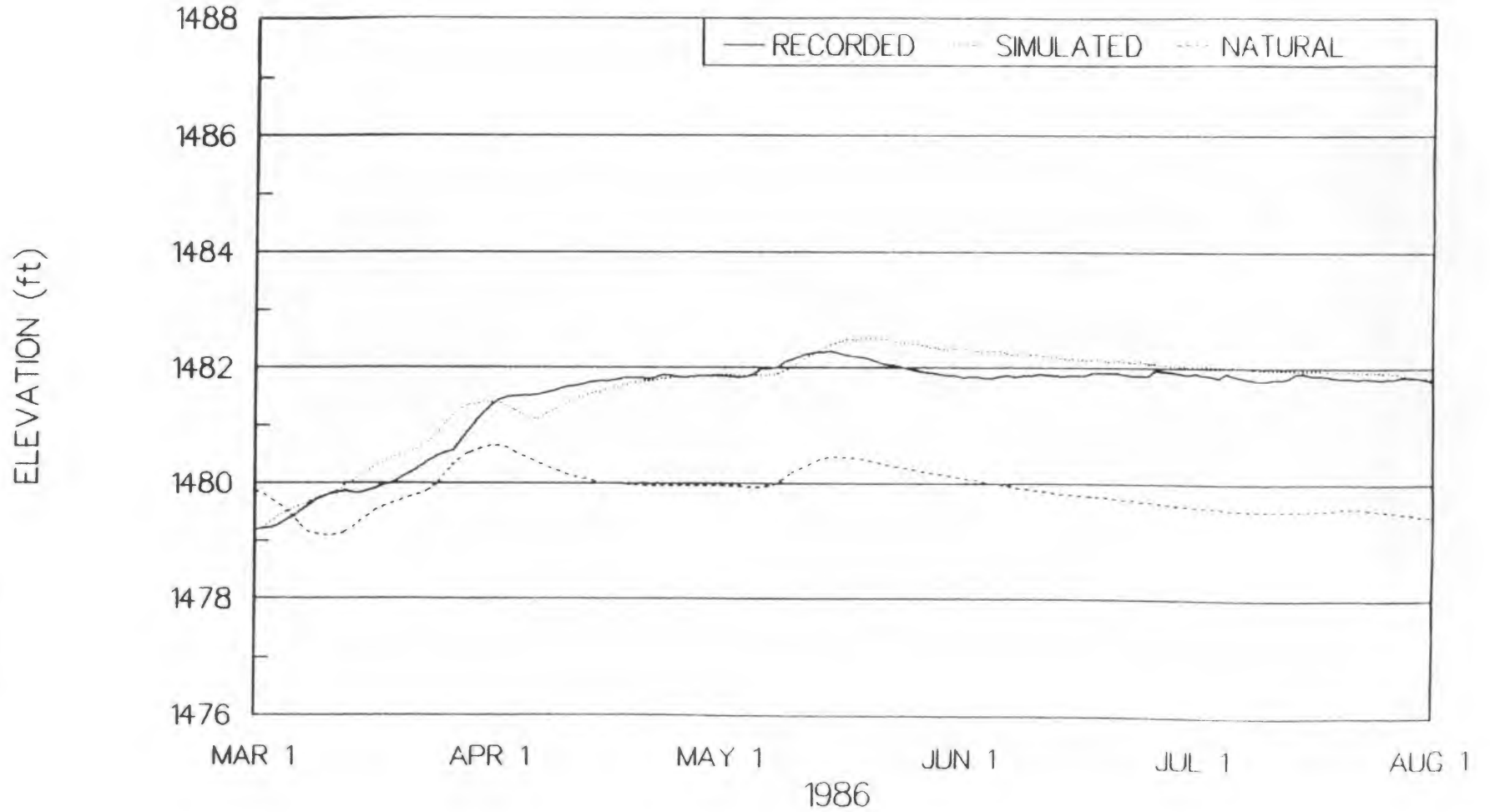


Figure 3.1-8

ROUND LAKE ELEVATION

1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

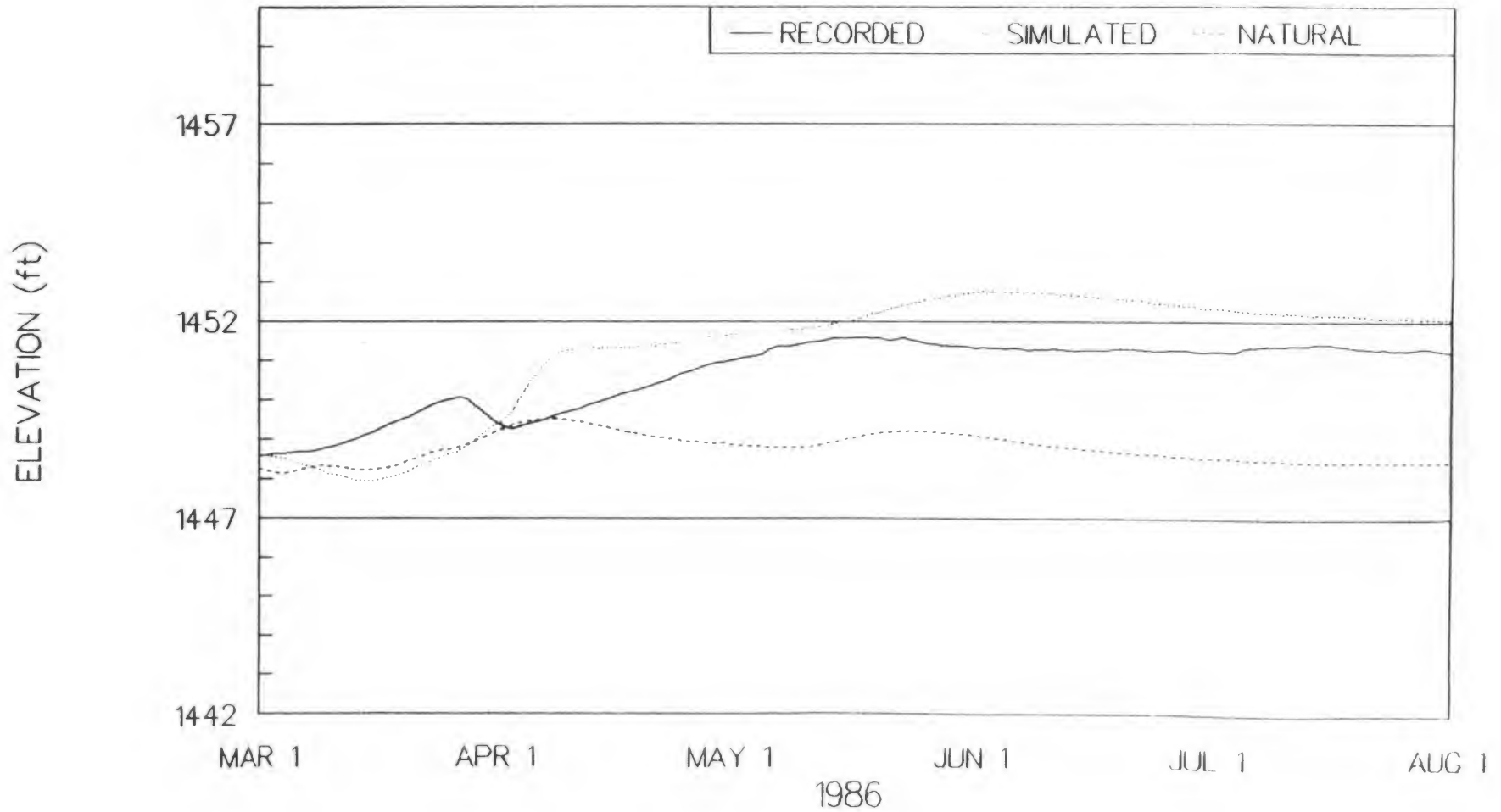


Figure 3.1-9

QU'APPELLE RIVER NEAR WELBY
1986 SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODIFIED MODEL

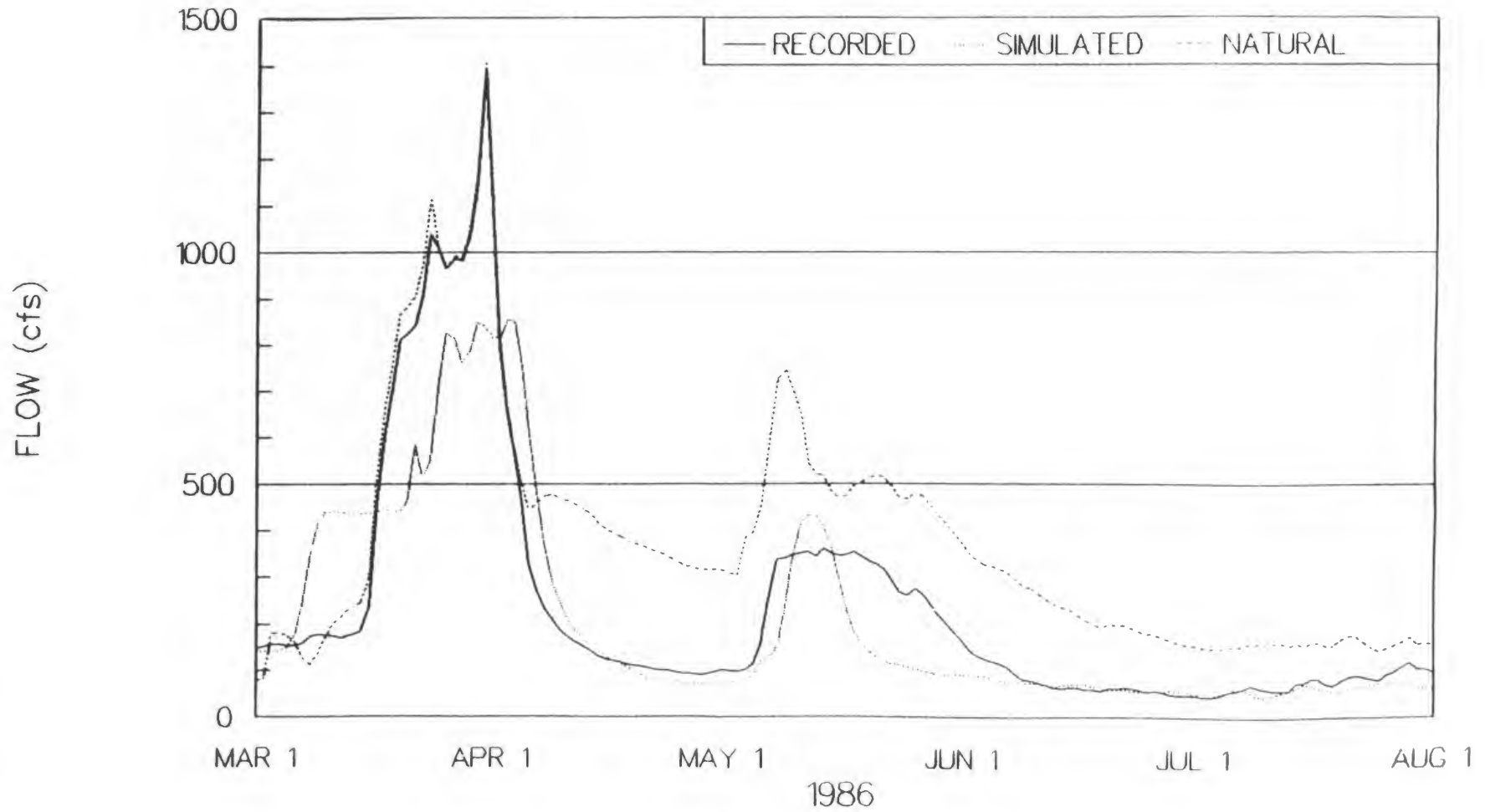


Figure 3.1-10

under both recorded and simulated conditions is included in the output of the spring and summer portion of the modified model as shown in Appendix E. The sum of the differences must be manually calculated and added to the natural flow volume of the Qu'Appelle River near Welby. An example of the required calculation is shown in Appendix E.

3.2 Comparison of Original Model and Modified Model Simulation Results

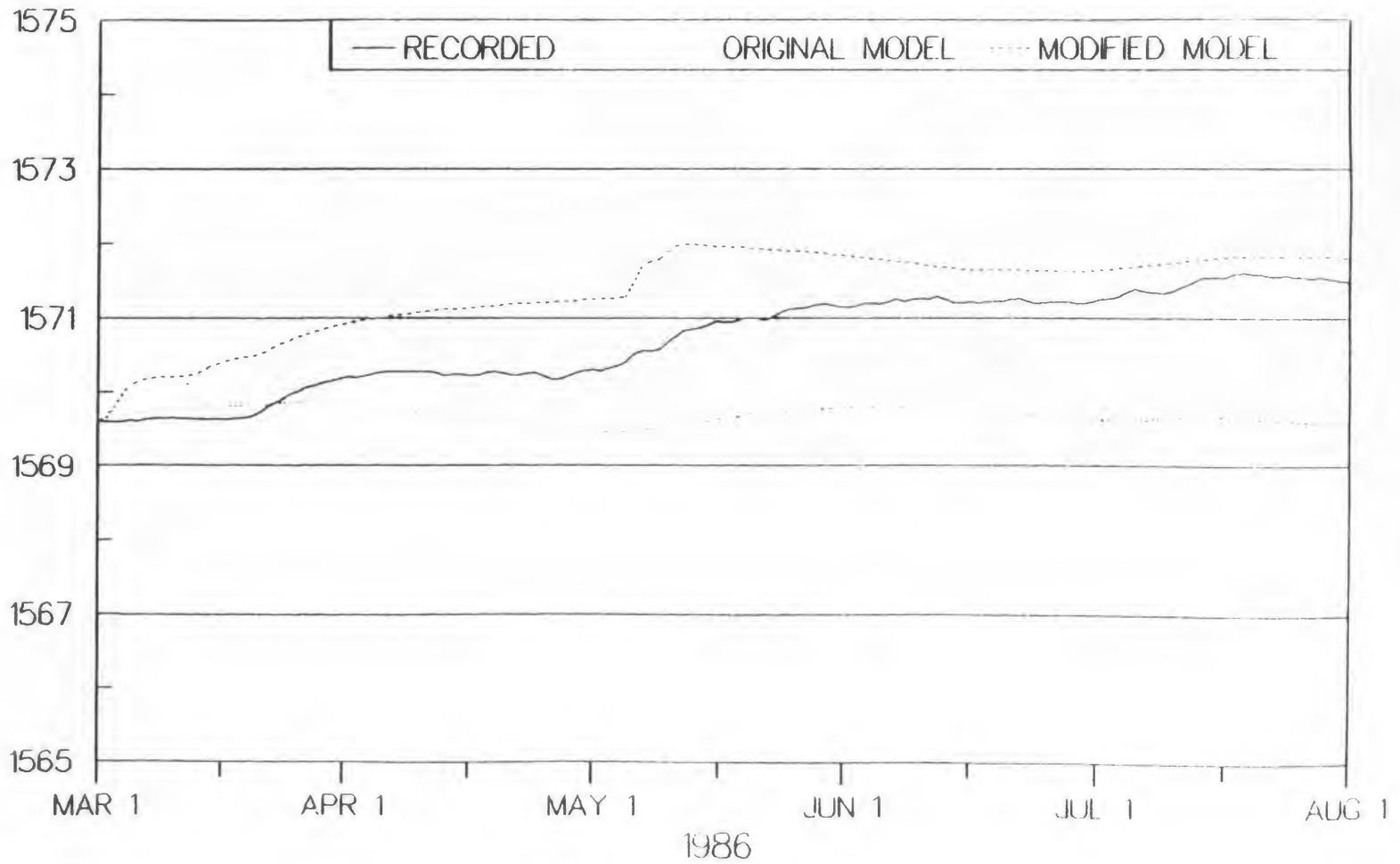
3.2.1 Existing Conditions Limb

Simulation results produced by the existing conditions limb of the spring and summer portions of both the original and the modified models for the year 1986 are plotted in Figures 3.2.1-1 to 3.2.1-6. These figures show the effect of the modifications made to the Qu'Appelle River Natural Flow Model on simulated elevations and streamflows for a number of locations along the Qu'Appelle River Valley. These figures show that lake elevations and streamflows simulated with the modified model are much closer to actual recorded data than those simulated by the original model.

Although a number of the modifications which were made to the model contribute to the improved simulation of existing conditions, the incorporation of ungauged local inflow and reservoir outlet control operation into the model were the main contributing factors. The original model simulated existing conditions without ungauged inflow and with the reservoir outlet structures permanently and completely open. The incorporation of these two factors into the natural flow

ECHO LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - EXISTING CONDITIONS LIMB



ELEVATION (ft)

Figure 3.2.1-1

QU'APPELLE RIVER BELOW KATEPWA LAKE

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - EXISTING CONDITIONS LIMB

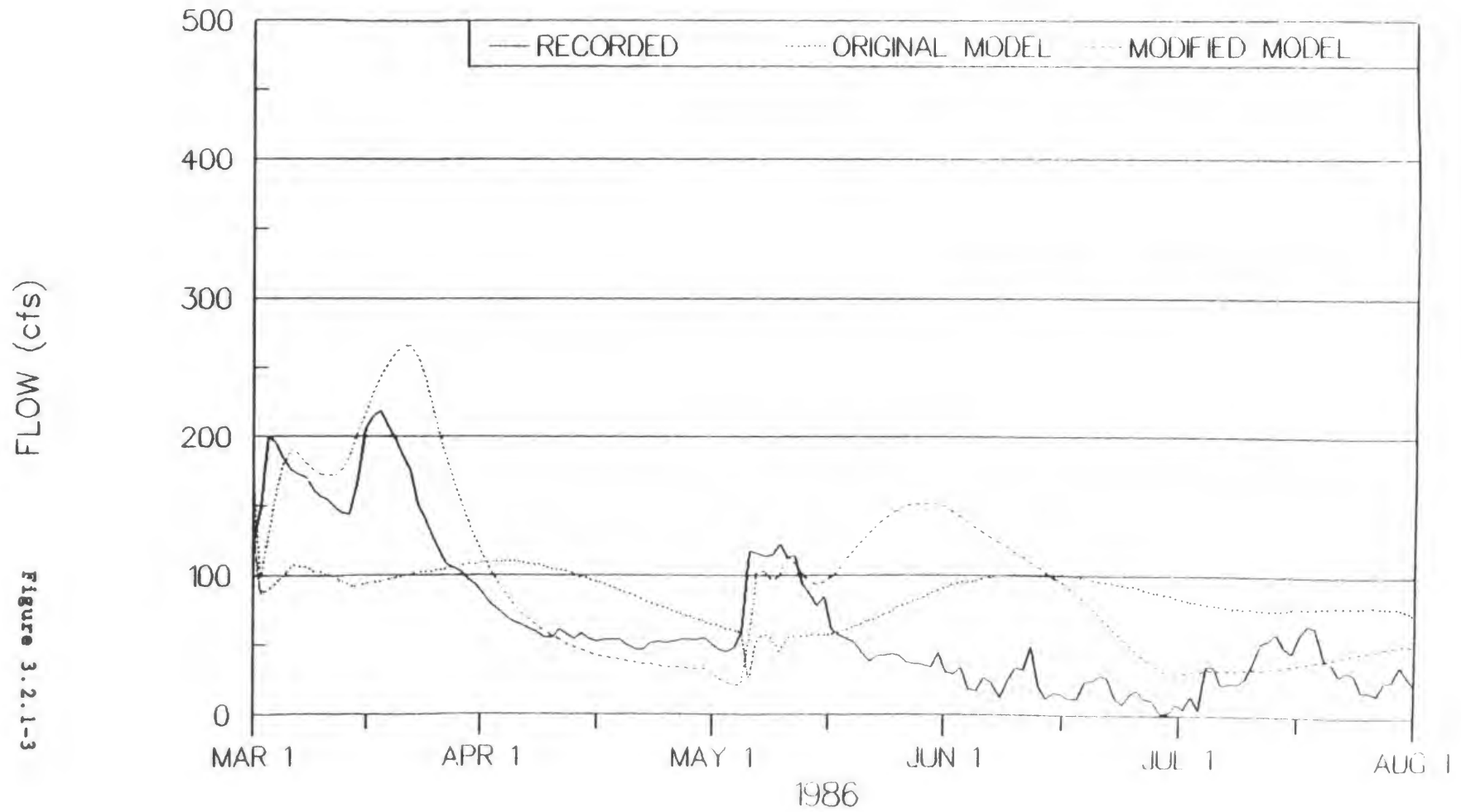
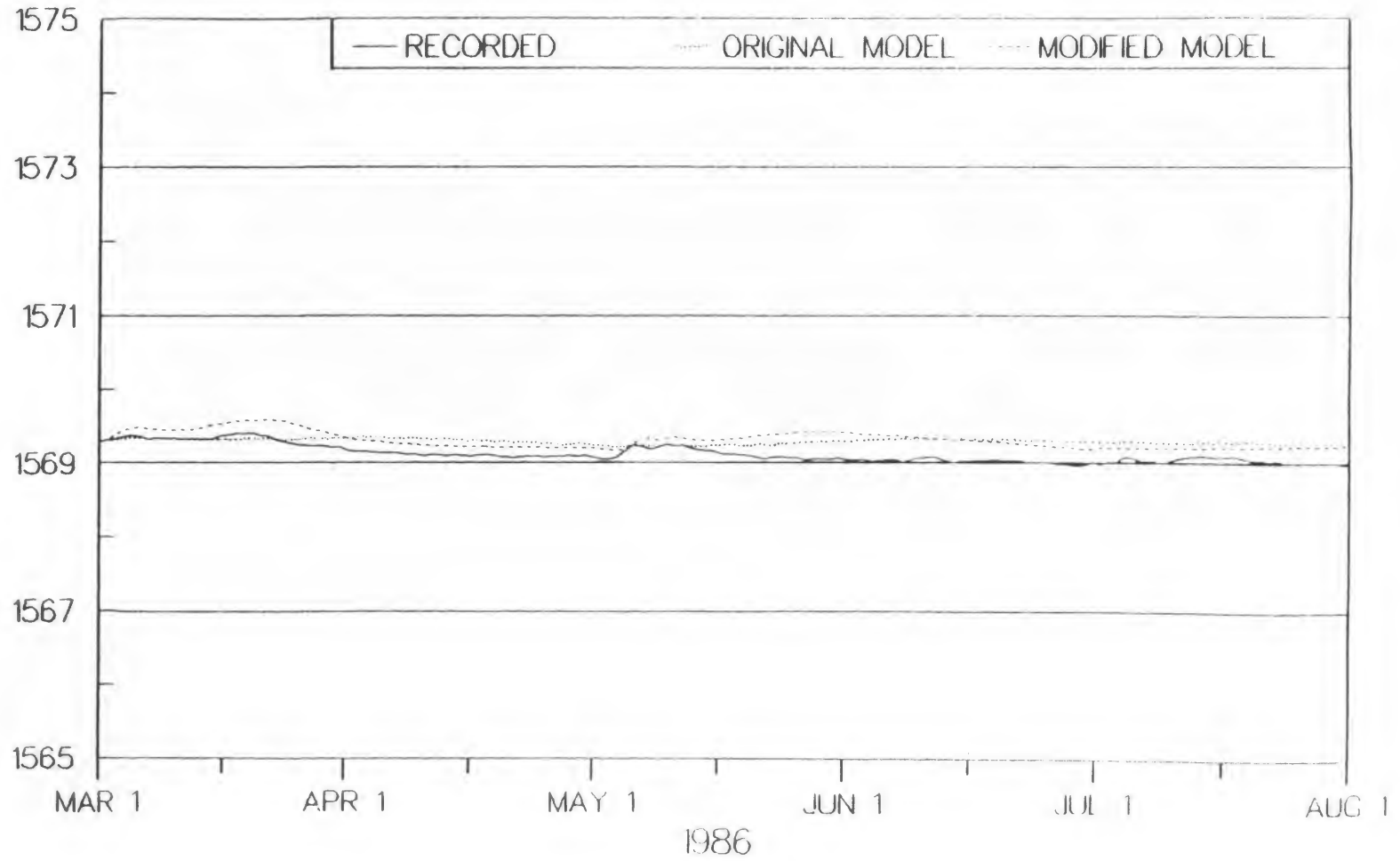


Figure 3.2.1-3

KATEPWA LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - EXISTING CONDITIONS LIMB



ELEVATION (ft)

Figure 3.2.1-2

CROOKED LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - EXISTING CONDITIONS LIMB

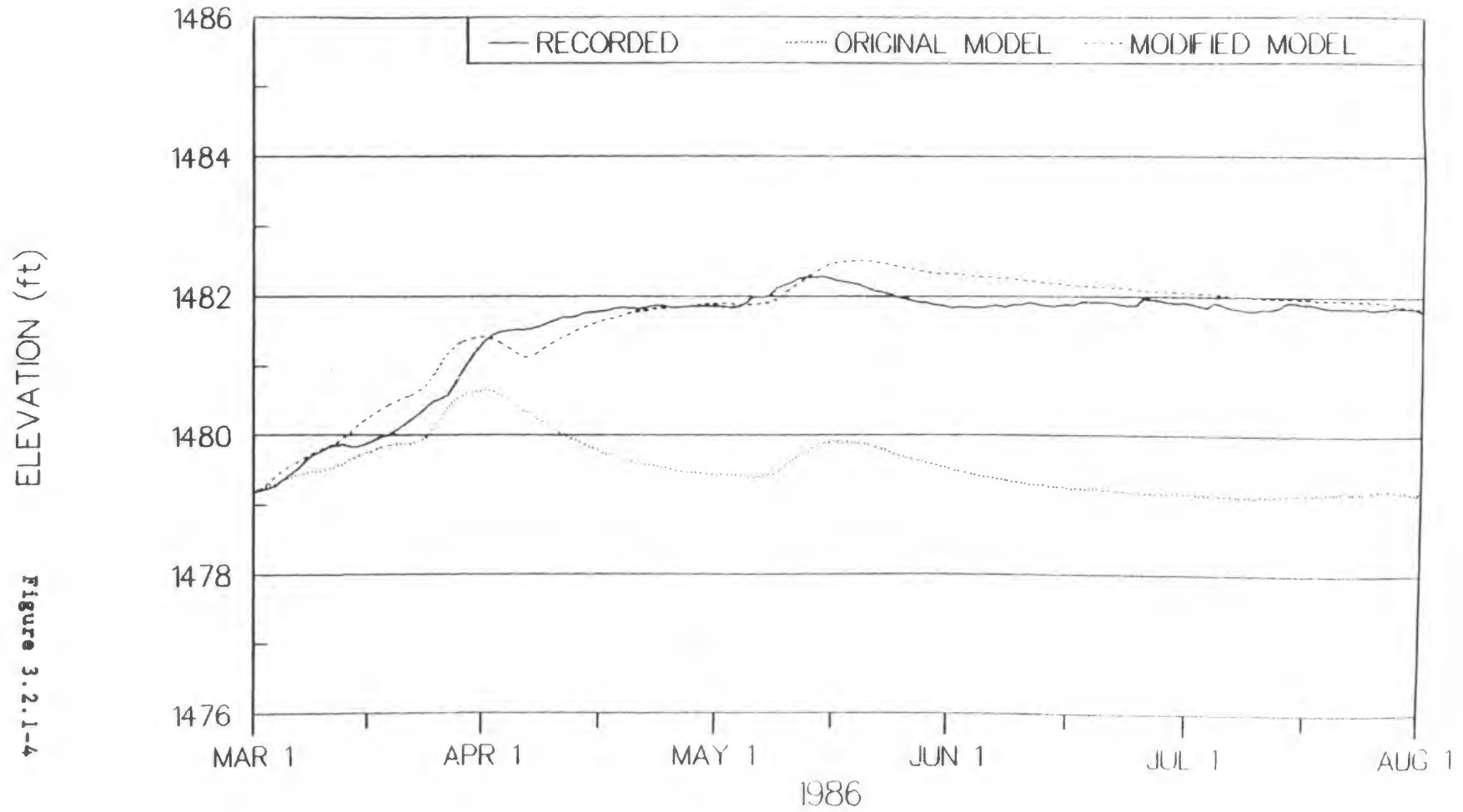
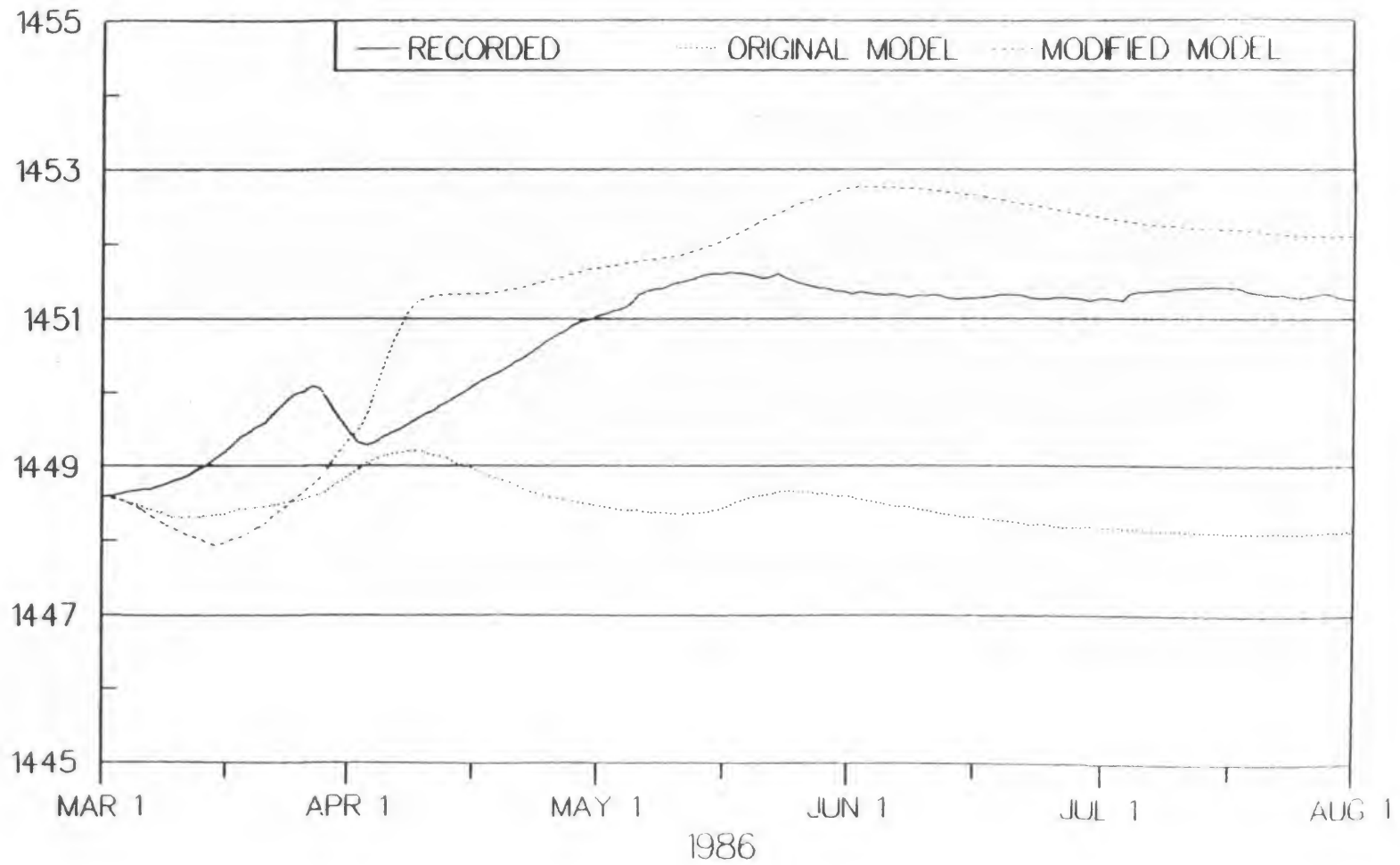


Figure 3.2.1-4

ROUND LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - EXISTING CONDITIONS LIMB

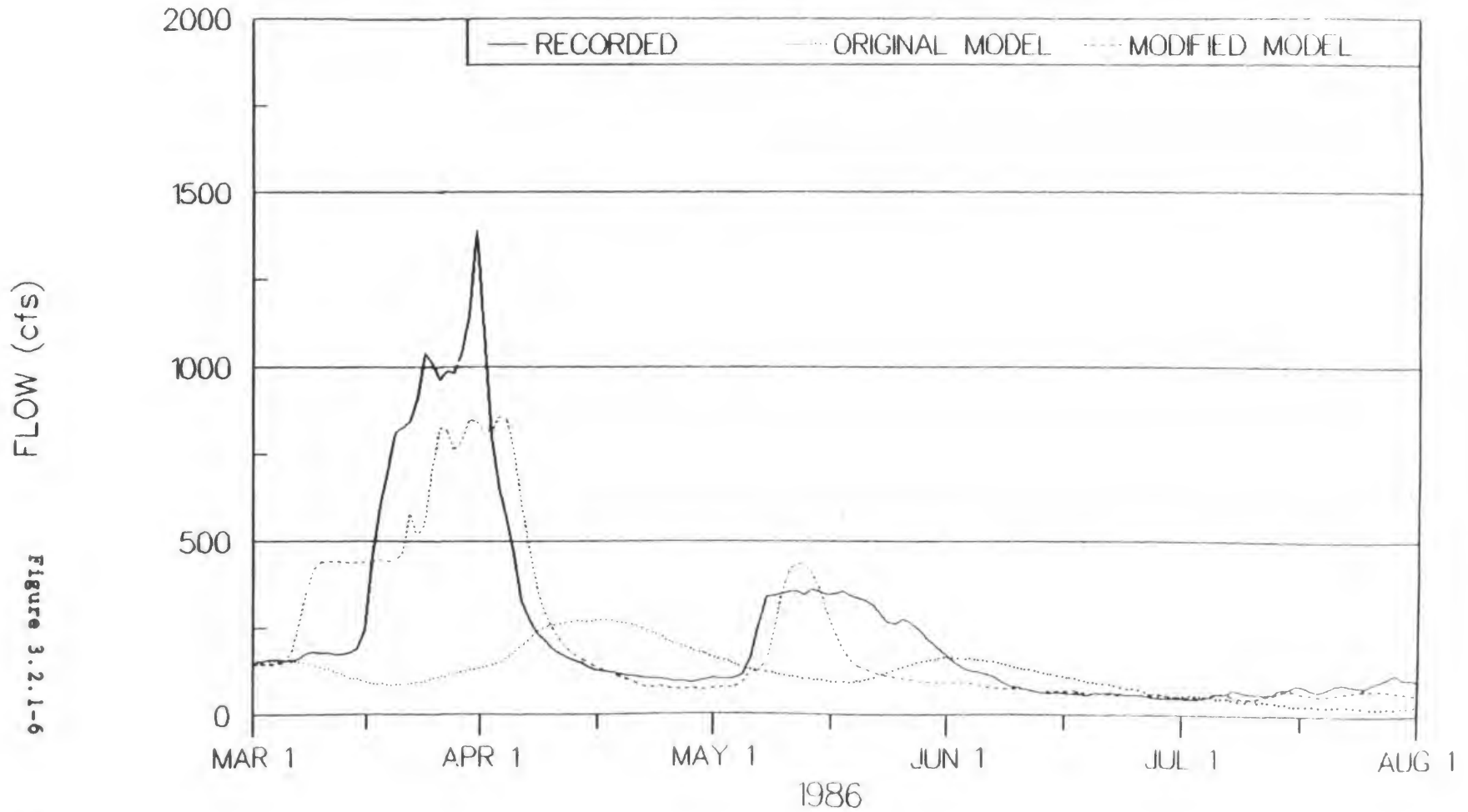


ELEVATION (ft)

Figure 3.2.1-5

QU'APPELLE RIVER NEAR WELBY

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - EXISTING CONDITIONS LIMB



FLOW (cfs)

Figure 3.2.1-6

model greatly improved the accuracy of the existing conditions flow limb, enabling one to meaningfully compare the results of the existing conditions limb to recorded data and evaluate the accuracy of the simulation.

3.2.2 Natural Conditions Limb

Simulation results produced by the natural flow limb of the spring and summer portions of both the original and the modified models for the year 1986 are plotted in Figures 3.2.2-1 to 3.2.2-10.

These figures show the cumulative effect of the modifications made to the Qu'Appelle River Natural Flow Model on the simulated natural lake elevations and streamflows. These figures show the modified model simulating natural lake elevations which are, in general, higher than those simulated with the original model.

BUFFALO POUND ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL -- NATURAL CONDITIONS LIMB

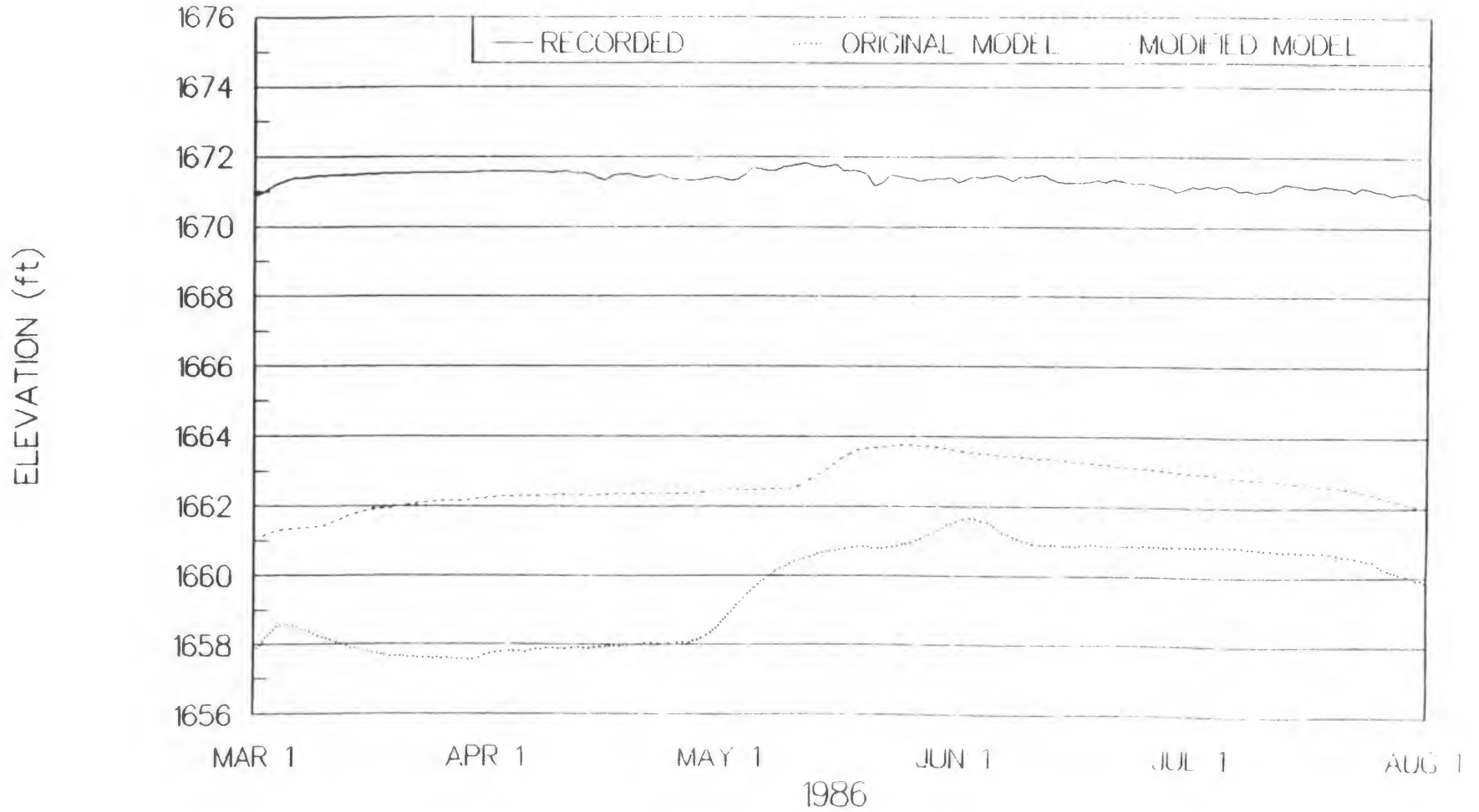


Figure 3.2.2-1

QU'APPELLE RIVER BELOW MOOSE JAW RIVER

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

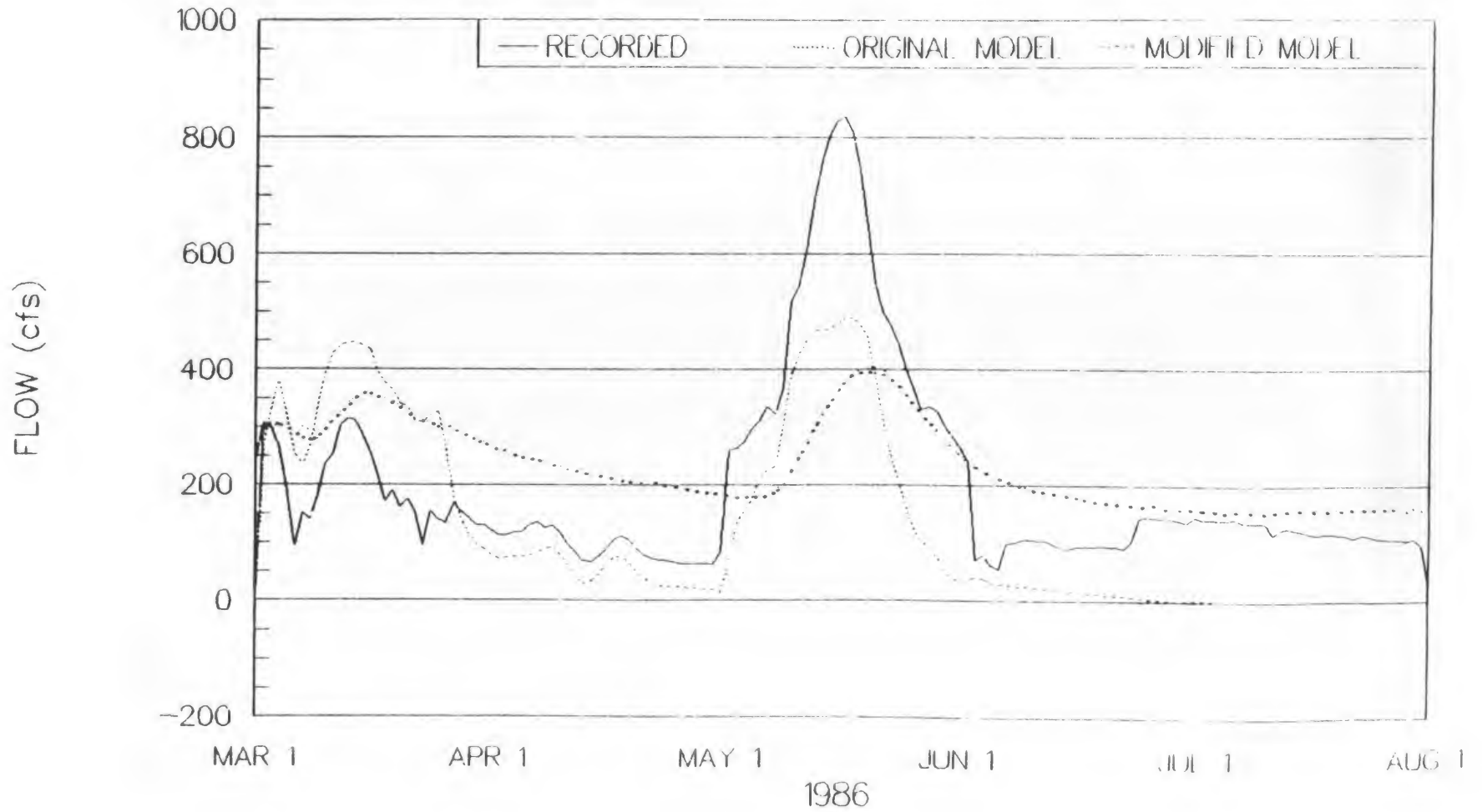


Figure 3.2.2-2

LAST MOUNTAIN LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

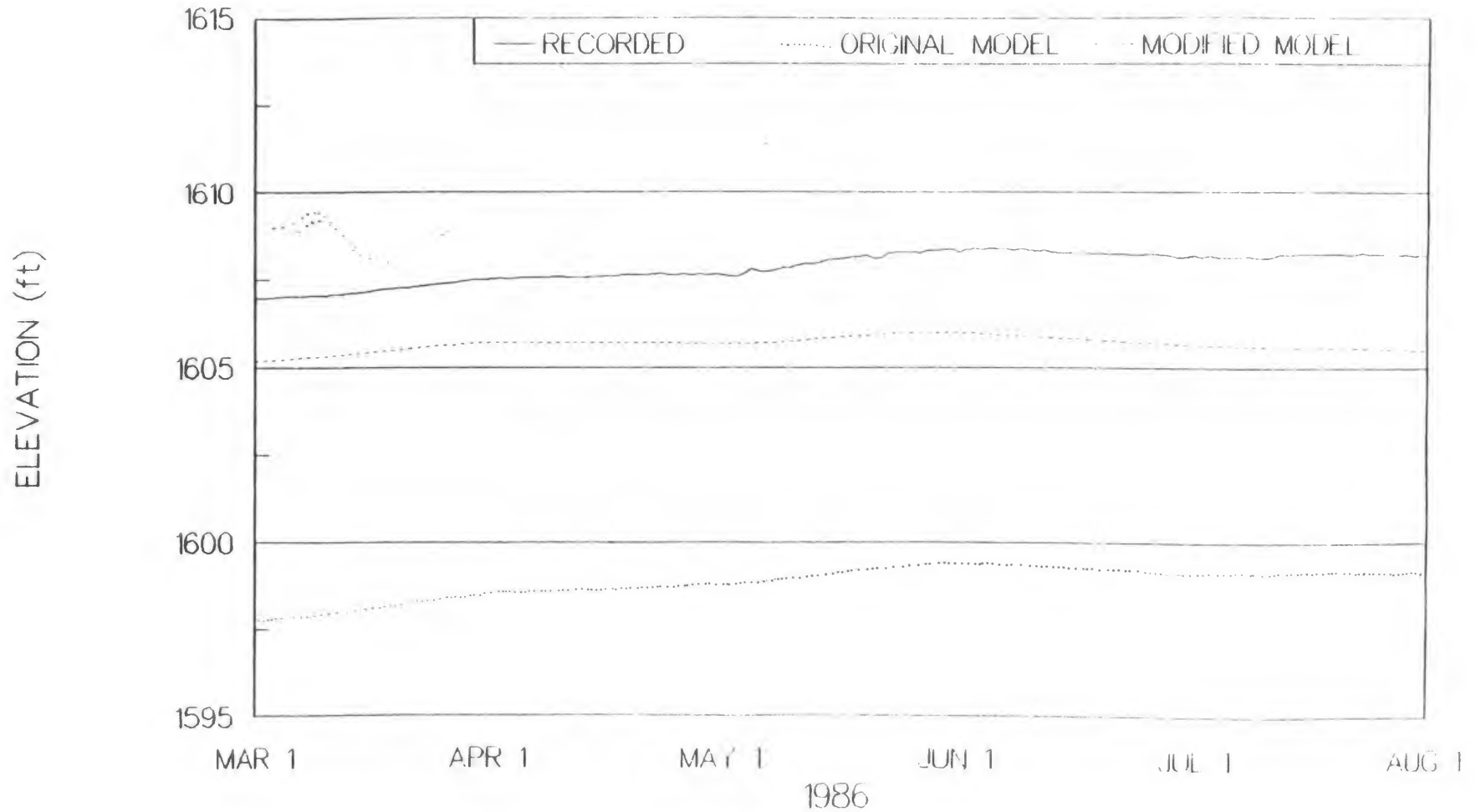


Figure 3.2.2-3

QU'APPELLE RIVER BELOW CRAVEN DAM

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

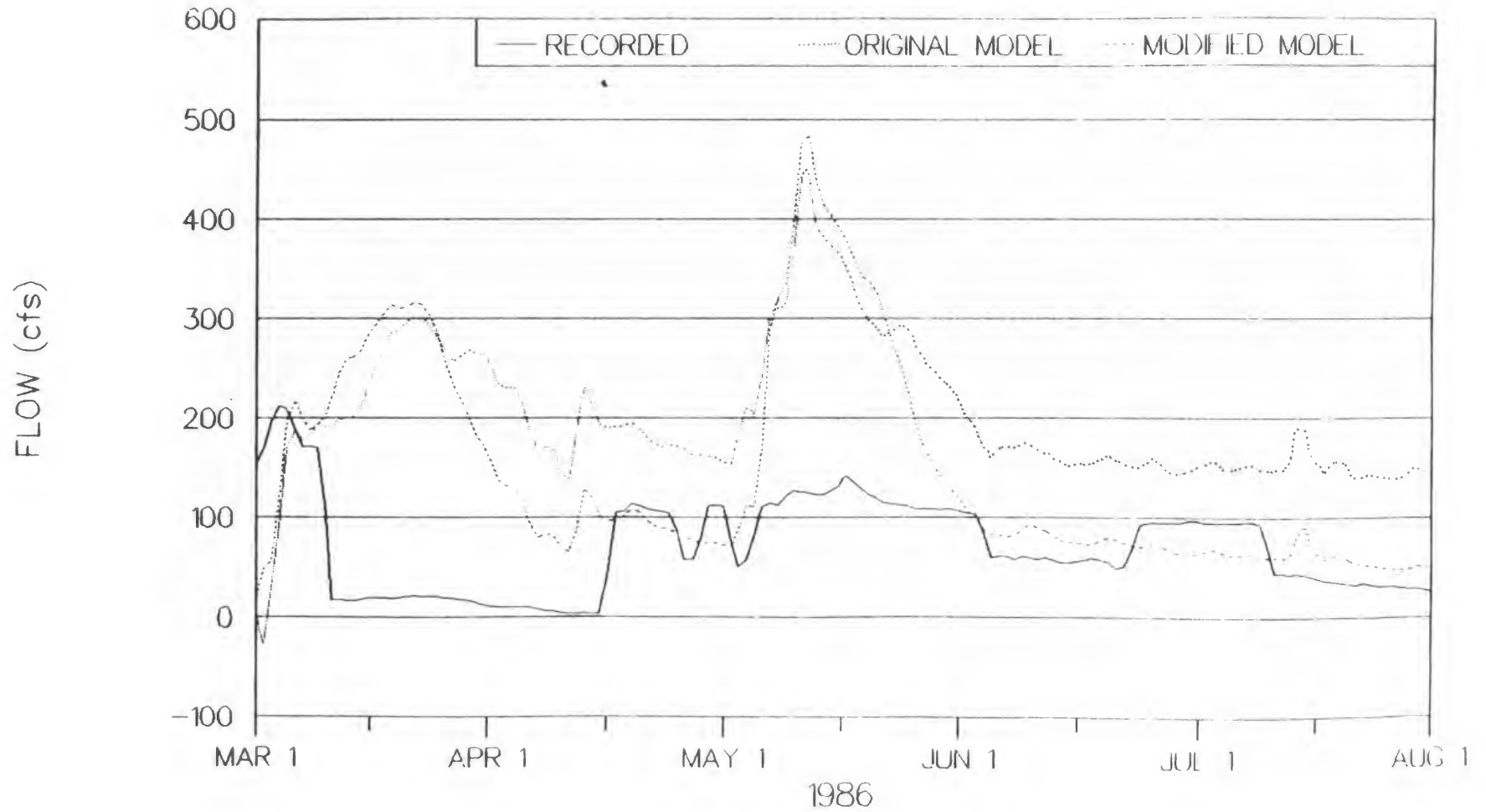


Figure 3.2.2-4

ECHO LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

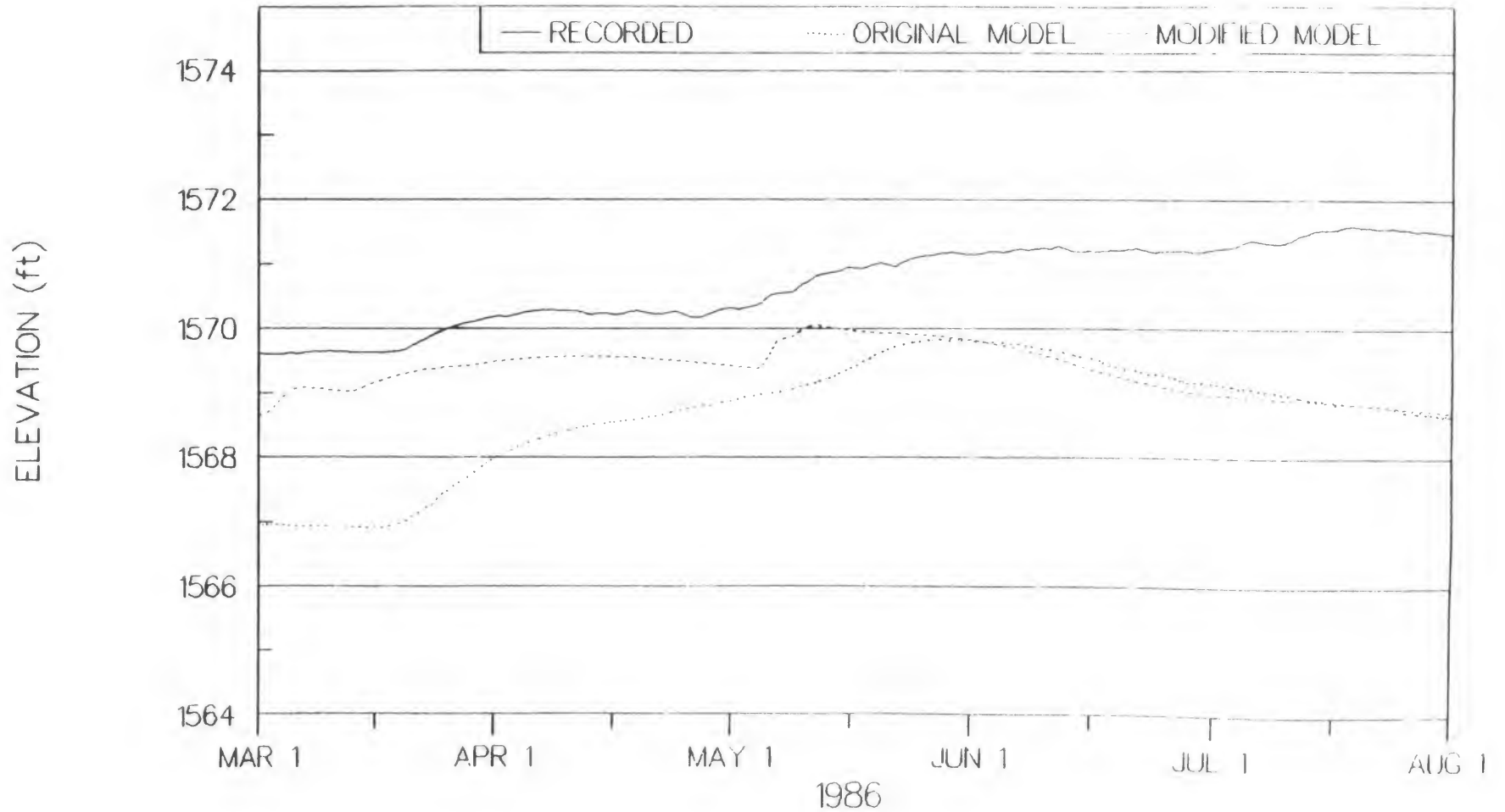


Figure 3.2.2-5

KATEPWA LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

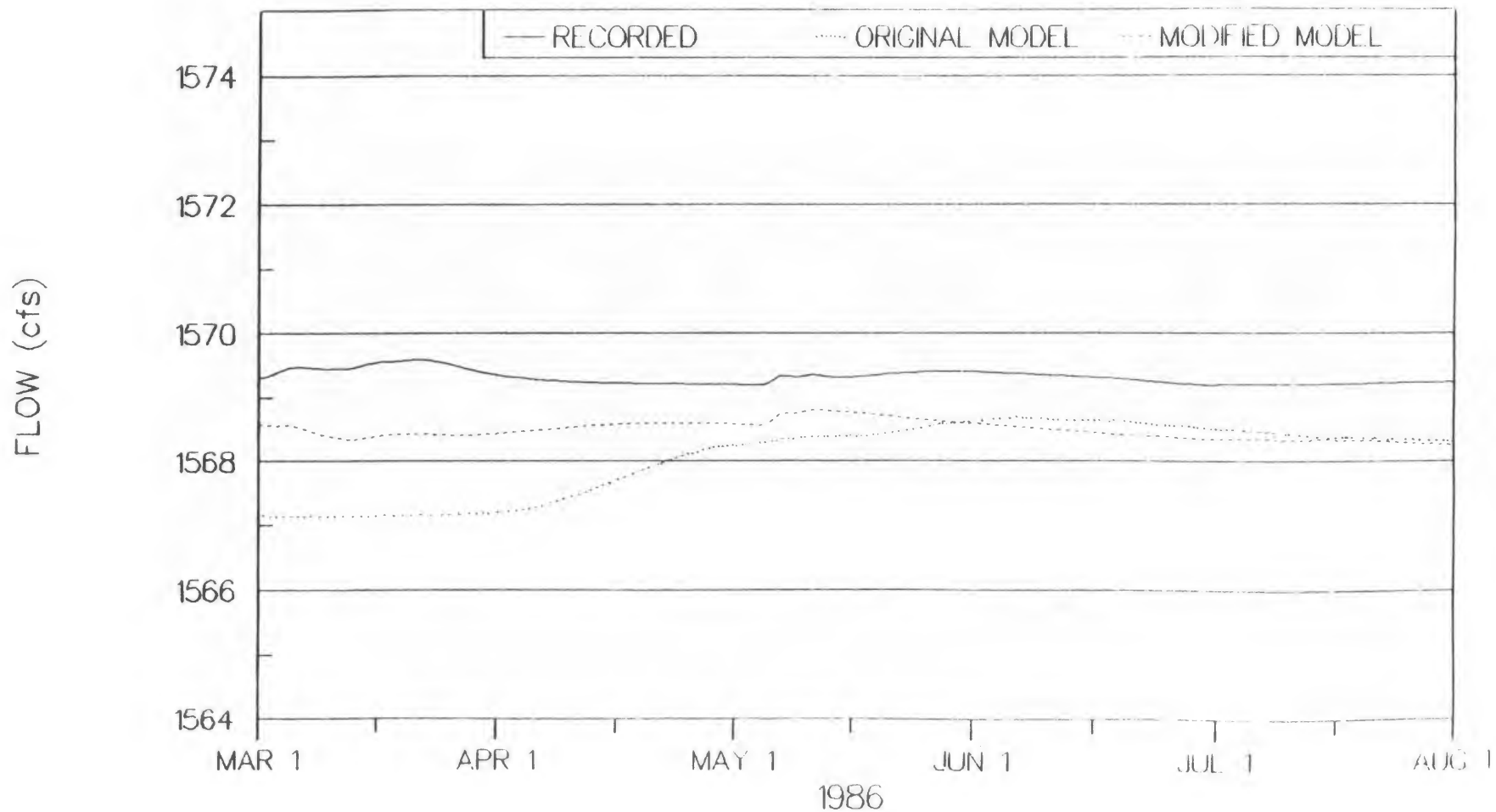


Figure 3.2.2-6

QU'APPELLE RIVER BELOW KATEPWA LAKE

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

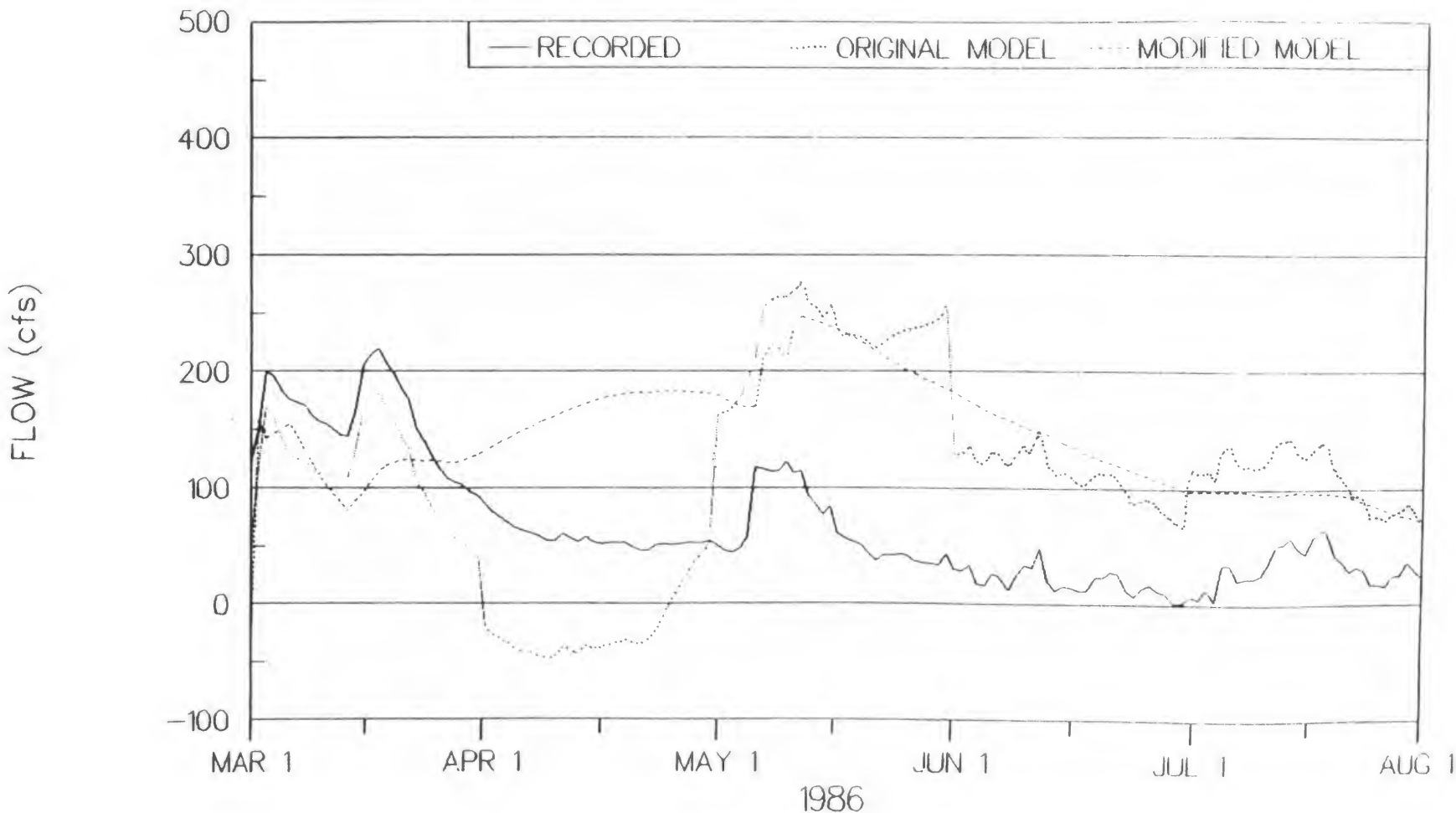


Figure 3.2.2-7

CROOKED LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

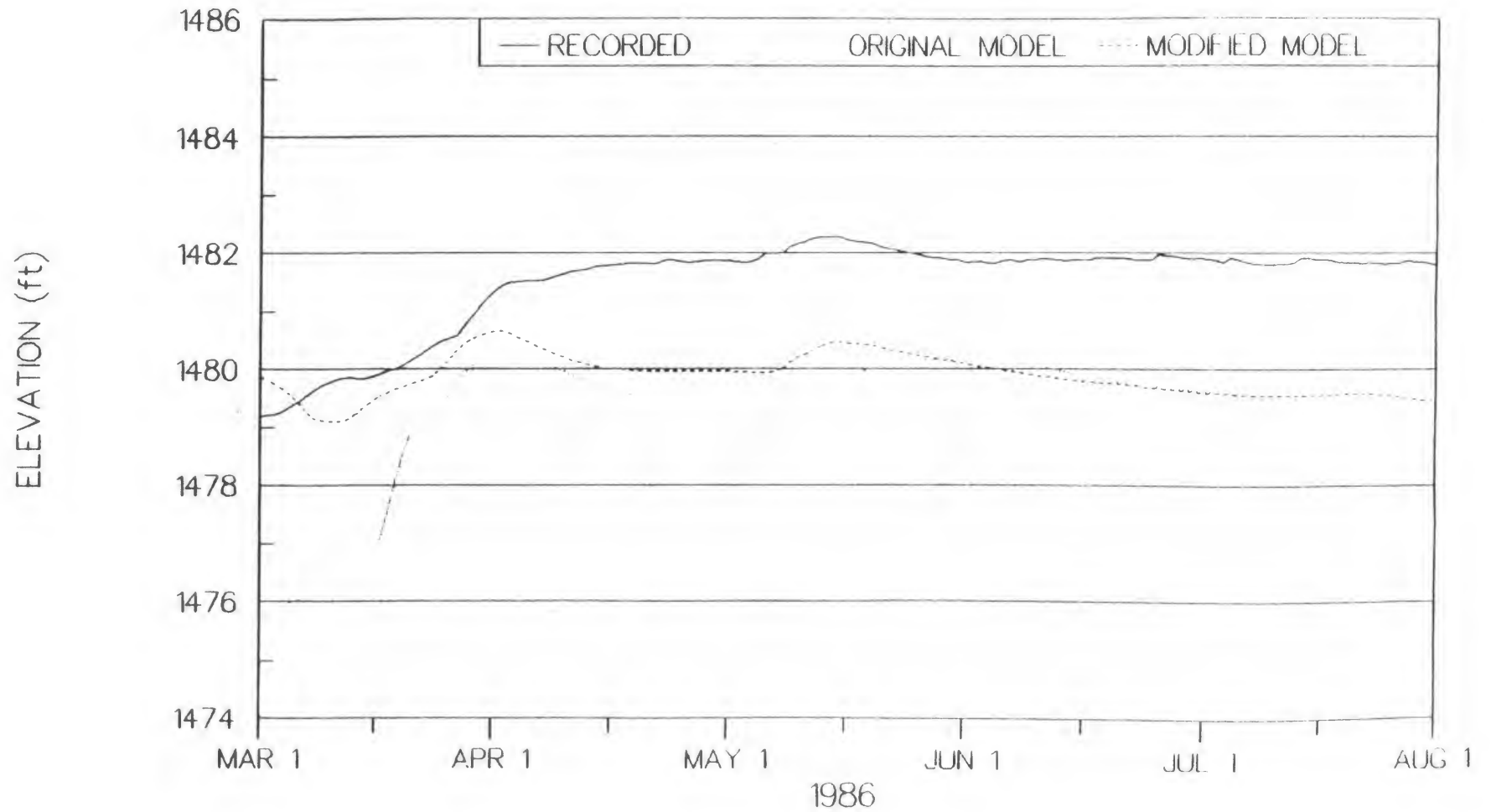


Figure 3.2.2-8

ROUND LAKE ELEVATION

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL -- NATURAL CONDITIONS LIMB

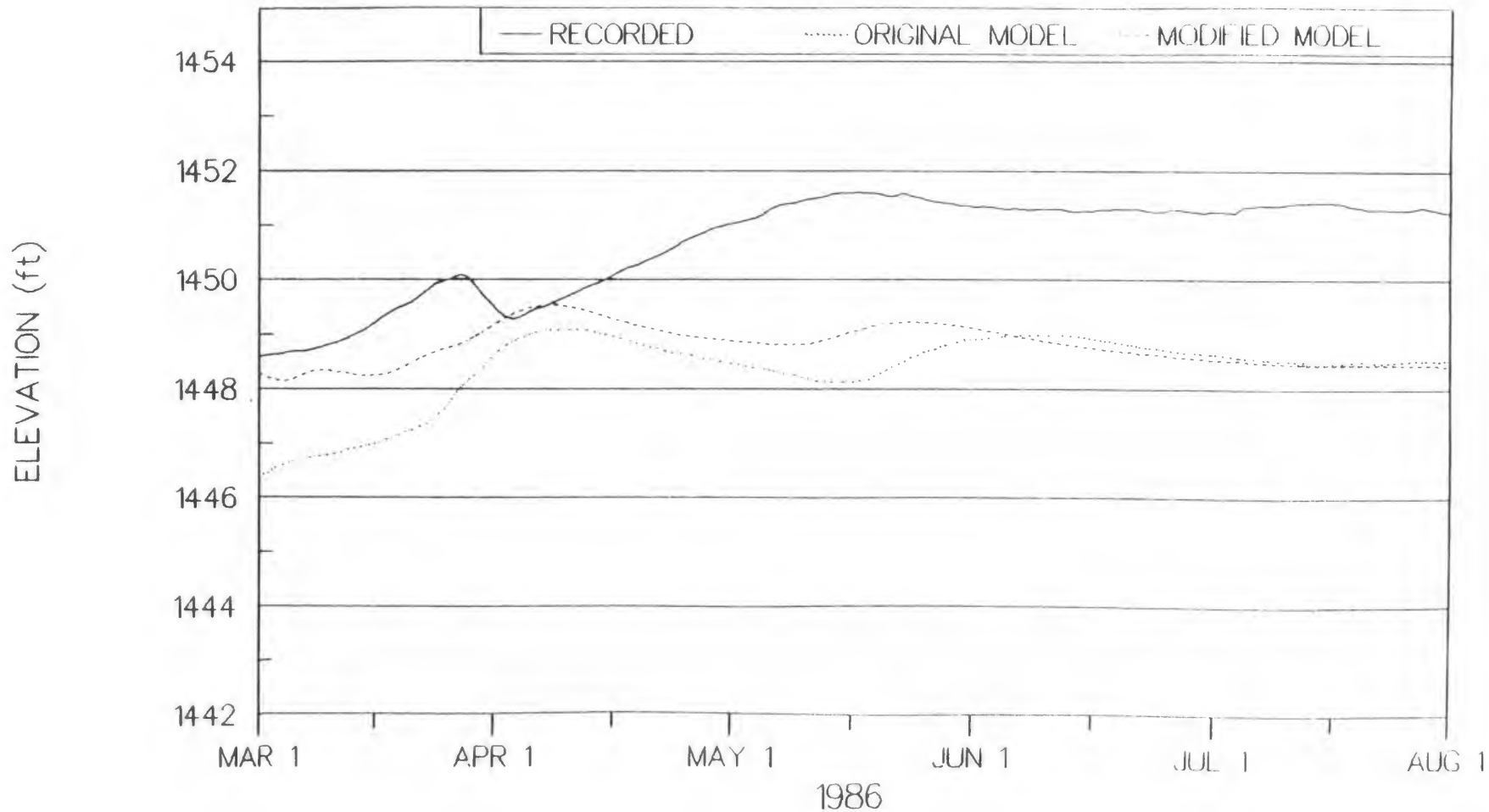


Figure 3.2.2-9

QU'APPELLE RIVER NEAR WELBY

COMPARISON OF ORIGINAL AND MODIFIED MODEL SIMULATION RESULTS
SPRING AND SUMMER PORTION OF MODEL - NATURAL CONDITIONS LIMB

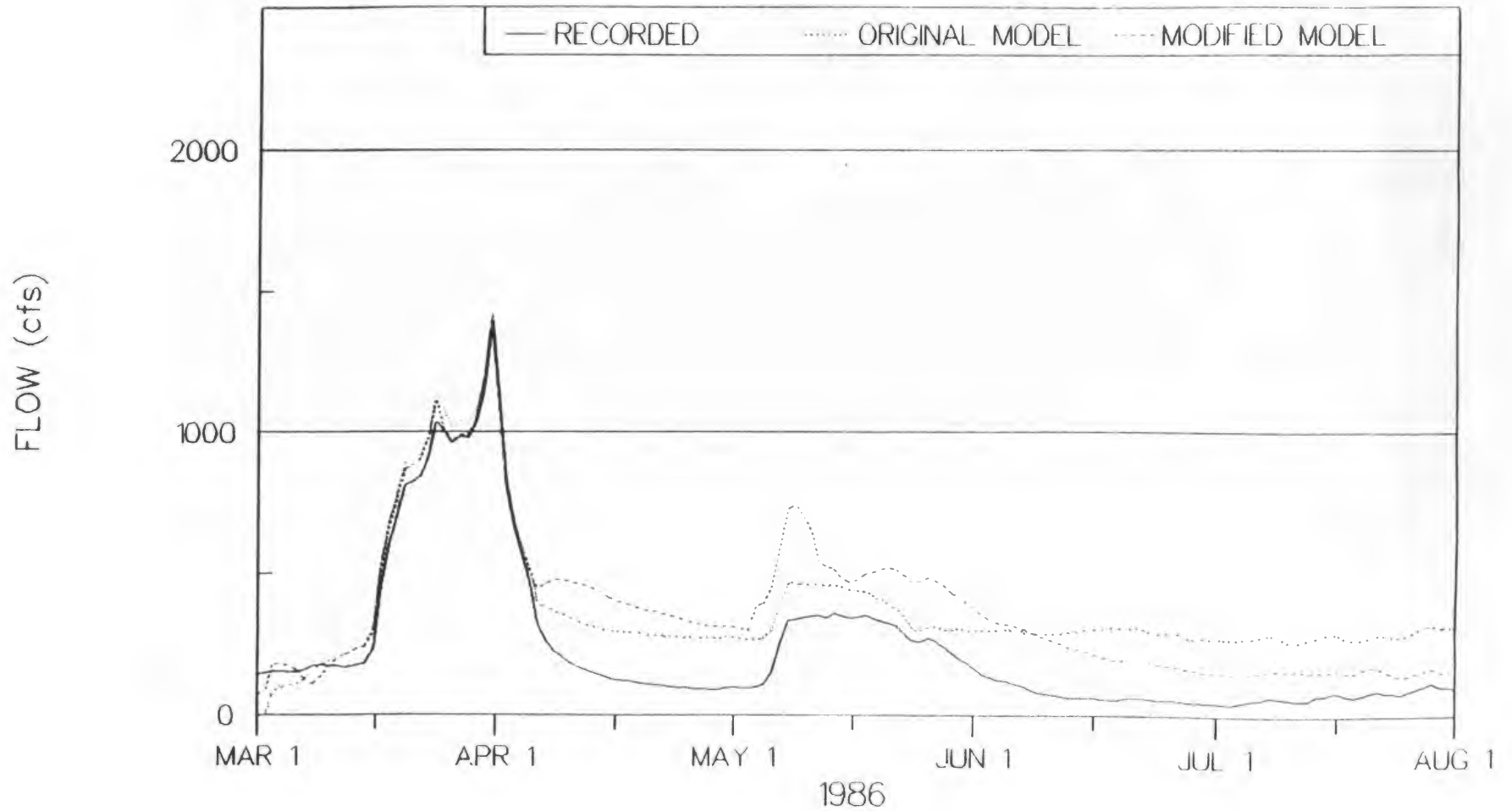


Figure 3.2.2-10

LAST MOUNTAIN LAKE

COMPARISON OF NATURAL ELEVATIONS SIMULATED WITH ORIGINAL AND MODIFIED NATURAL FLOW MODELS

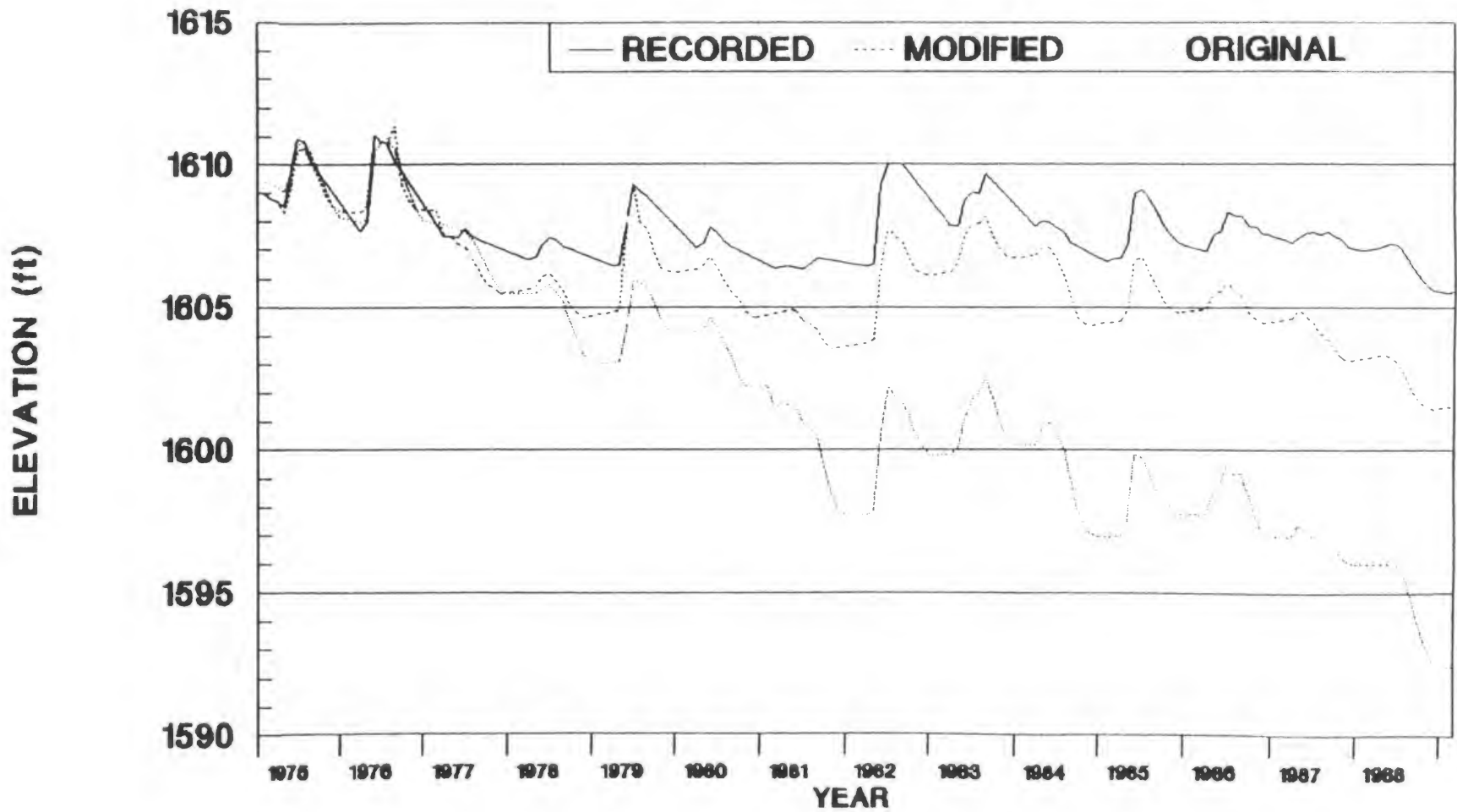


Figure 3.2.2-11

3.2.3 Fall and Winter Segment

Simulation results produced by the fall and winter segment of the modified model are compared with simulation results produced by the original model and with recorded data in Figures 3.2.3-1 & 3.2.3-2.

Figure 3.2.3-1, a plot of Last Mountain Lake elevations, shows the modified model simulates a higher natural lake elevation than that simulated by the original model. This figure also shows the natural elevation of Last Mountain Lake simulated by the modified model dropping at a slower rate than that simulated by the original model. This is the result of the 31 cfs groundwater flow into Last Mountain Lake which is incorporated into the fall and winter segment of the modified model and the revised outflow relationship table.

Figure 3.2.3-2, a plot of the flow in the Qu'Appelle River near Welby for the August/1986 to March/1987 period shows the cumulative affect of the modifications made to the model on the fall and winter simulated natural flow.

One of the major concerns was the low and steadily dropping natural lake elevation of Last Mountain Lake simulated by the original model. Figure 3.2.2-11 shows the natural elevation of Last Mountain Lake simulated with the original and with the modified model for the years 1975 to 1988. This figure shows the natural lake elevation of Last

LAST MOUNTAIN LAKE

SIMULATED NATURAL AND RECORDED ELEVATION
AUGUST 1 1986 TO MARCH 1 1987

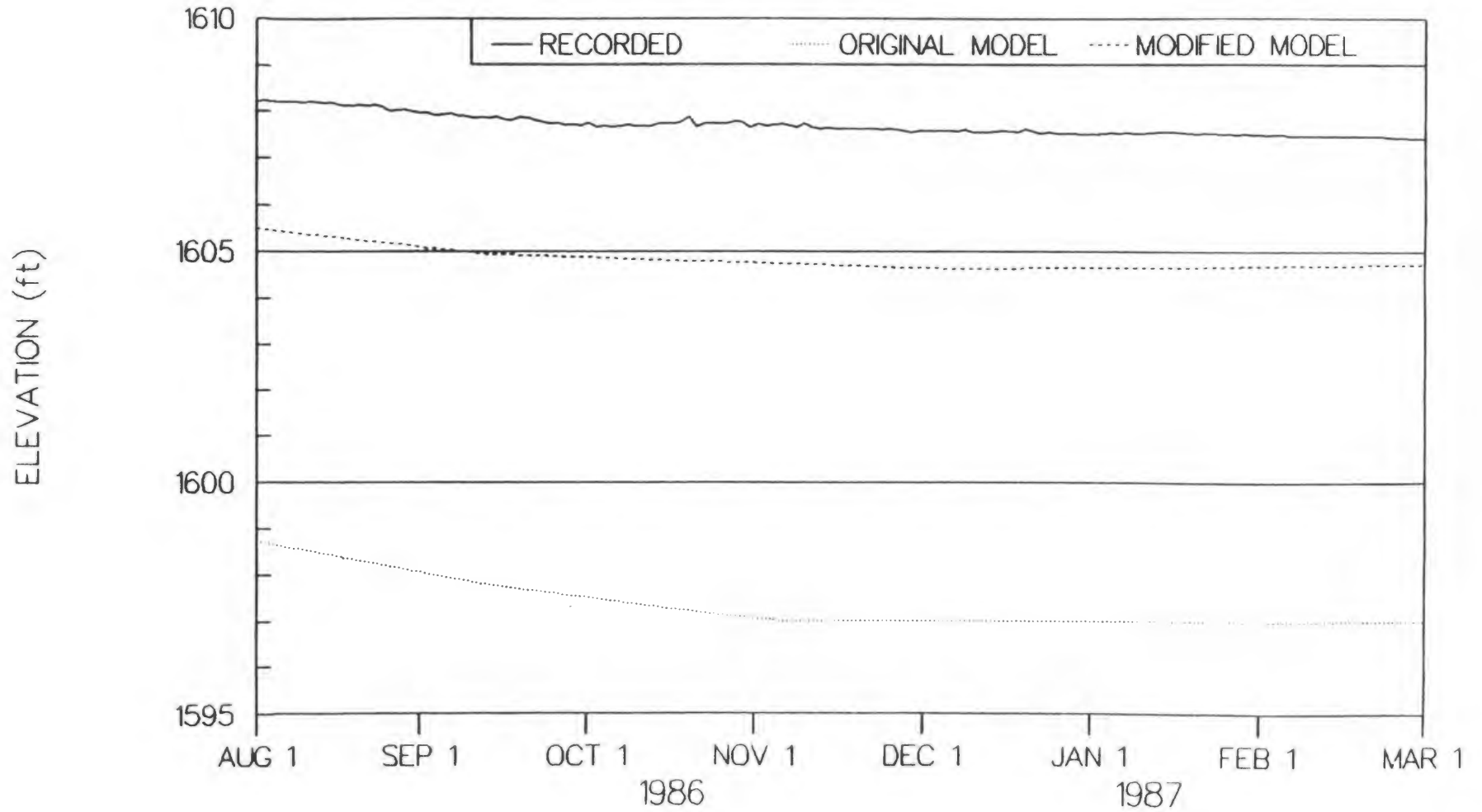


Figure 3.2.3-1

QU'APPELLE RIVER NEAR WELBY

SIMULATED NATURAL AND RECORDED FLOW
AUGUST 1 1986 TO MARCH 1 1987

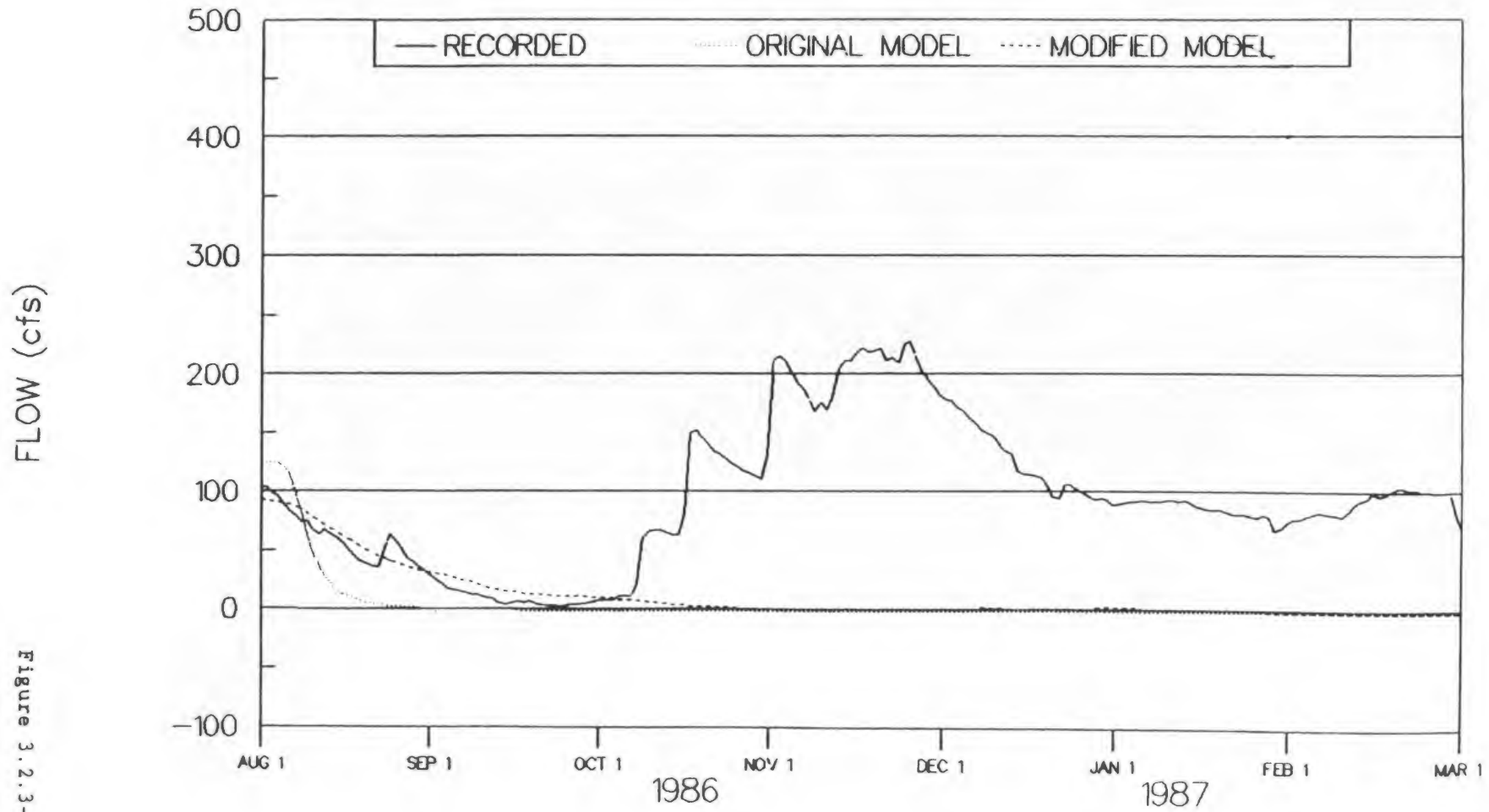


Figure 3.2.3-2

Mountain Lake simulated with the modified model, is much higher than that simulated with the original model.

Although the difference in the simulated natural elevation of Last Mountain Lake is the cumulative result of a number of changes which were made to the model, the difference is mainly attributed to the modified overflow table (CF20), compensation for lower lake evaporation under natural conditions, and compensation for groundwater inflow in winter.

3.3 Apportionment Flow at Saskatchewan/Manitoba Boundary

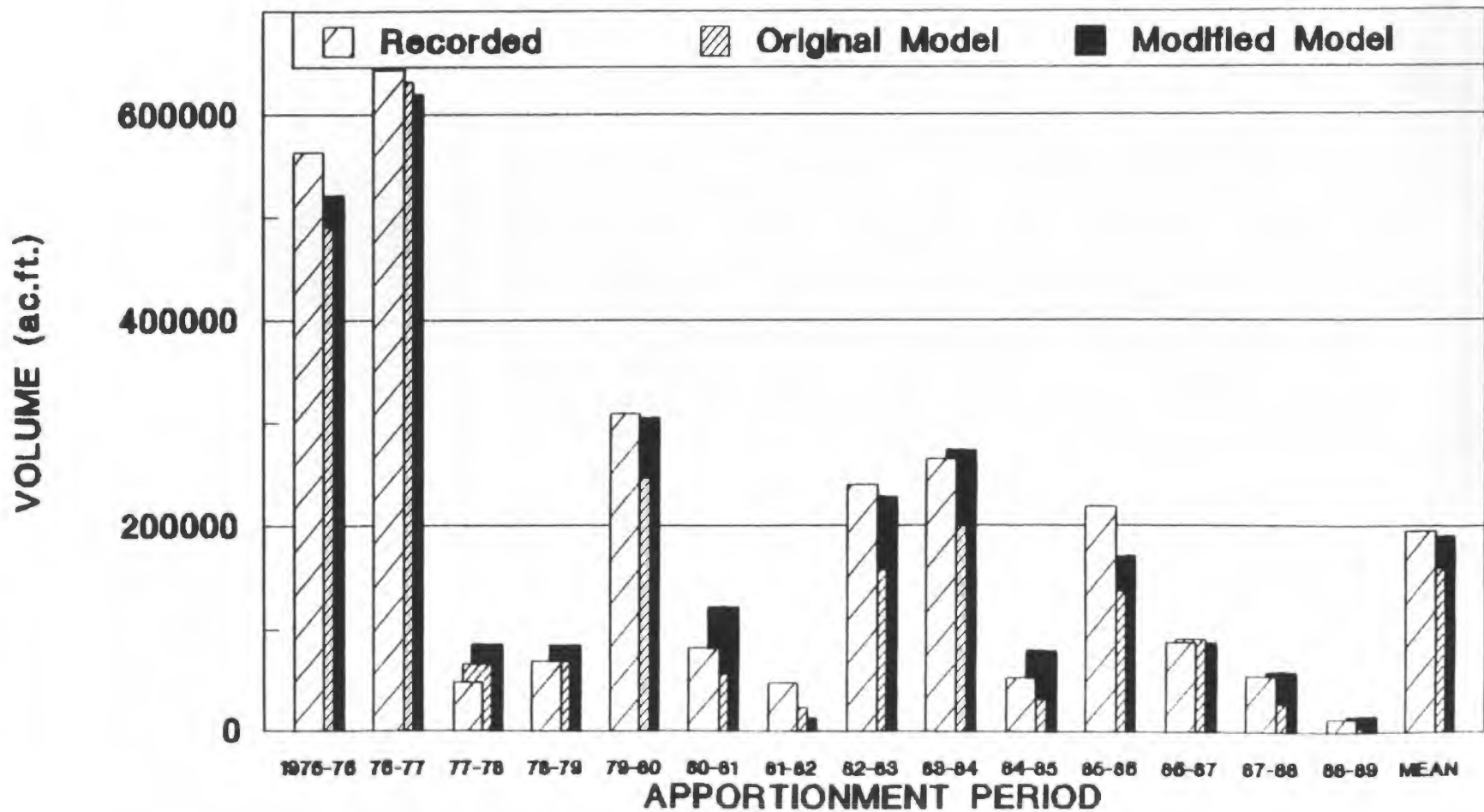
Apportionment period natural flow volumes for the Qu'Appelle River near Welby, simulated by the original and modified natural flow models for the apportionment period 1975/76 to 1988/89, are shown in Figure 3.3-1. Also shown in this figure, for comparison purposes, are the flow volumes recorded at the hydrometric station 05JM001 - Qu'Appelle River near Welby for these same apportionment periods. The apportionment period is considered to be from April 1 to March 31 of the following year.

Figure 3.3-1 shows that the modified model simulated higher apportionment period flow volumes than those simulated by the original model. The mean apportionment period volume simulated by the original Qu'Appelle River Natural Flow Model for the 1975/76 to 1988/89 period was 159850 acre-feet (197170 dam³); the mean apportionment period volume simulated by the modified model for the same period was 190830 acre-feet (235390 dam³); 30980 acre-feet (38210 dam³) or 19 percent higher.

QU'APPELLE RIVER NEAR WELBY

APPORTIONMENT PERIOD FLOW VOLUMES

COMPARISON OF RECORDED AND SIMULATED NATURAL VOLUMES

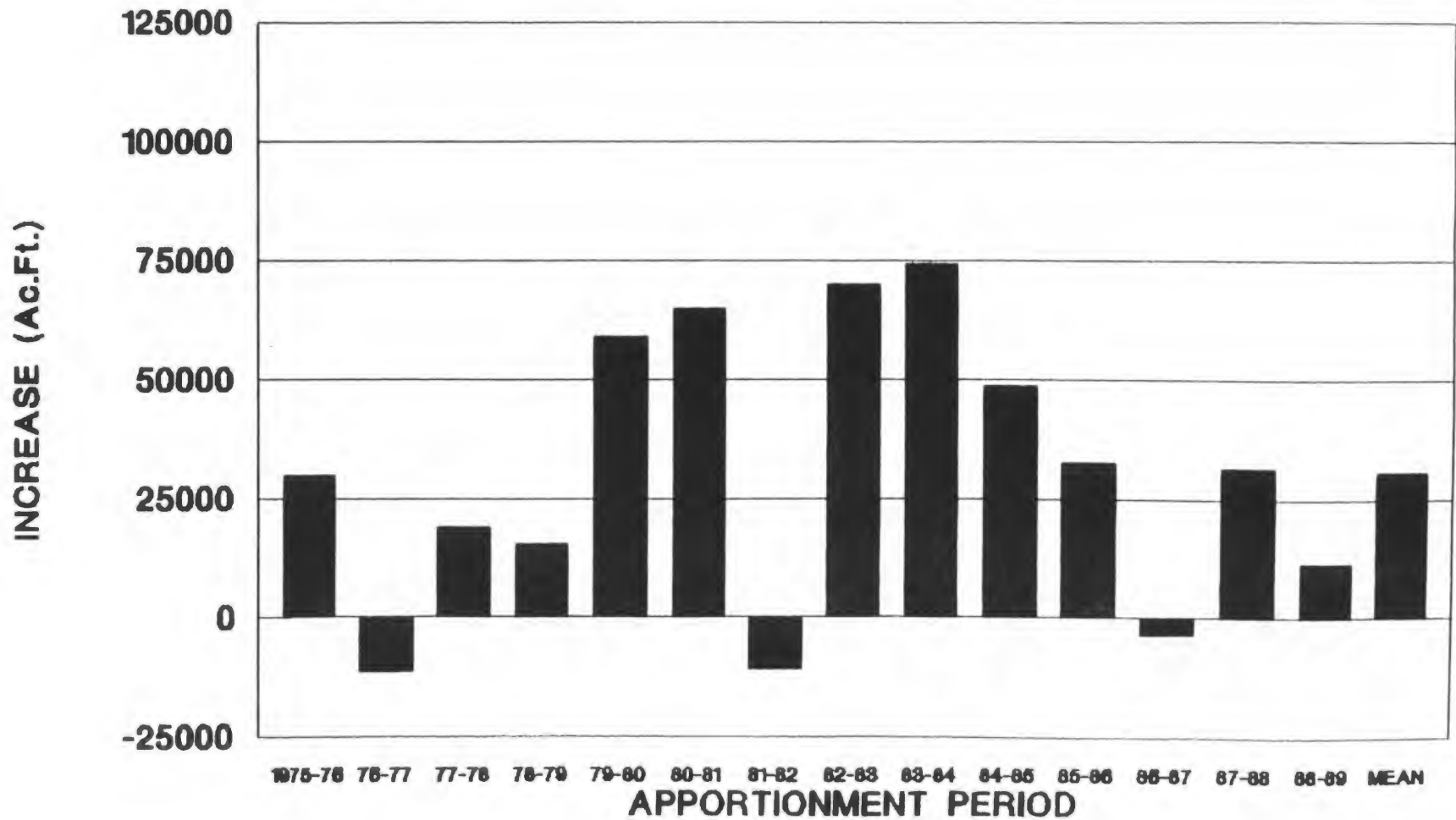


Note: Apportionment Period is April 1 to March 31

Figure 3.3-1

QU'APPELLE RIVER NEAR WELBY

INCREASE IN APPORTIONMENT PERIOD NATURAL FLOW



Note: Apporportionment Period is April 1 to March 31

The increase in the apportionment period natural flow volume estimated at the Saskatchewan/Manitoba Boundary is plotted in Figure 3.3-2. This figure shows the modified model simulated higher apportionment period flow volumes in all but three periods. In reviewing the original model output results in an effort to determine the reason for these three anomalies, it was discovered that in two of the periods in which the modified estimation was lower, the original model had to be run in a number of segments. Although the exact reason for this could not be determined, the fact that the run had to be split up is an indication that problems were experienced with the original model, thus casting doubts on the accuracy of the original model results for these two periods.

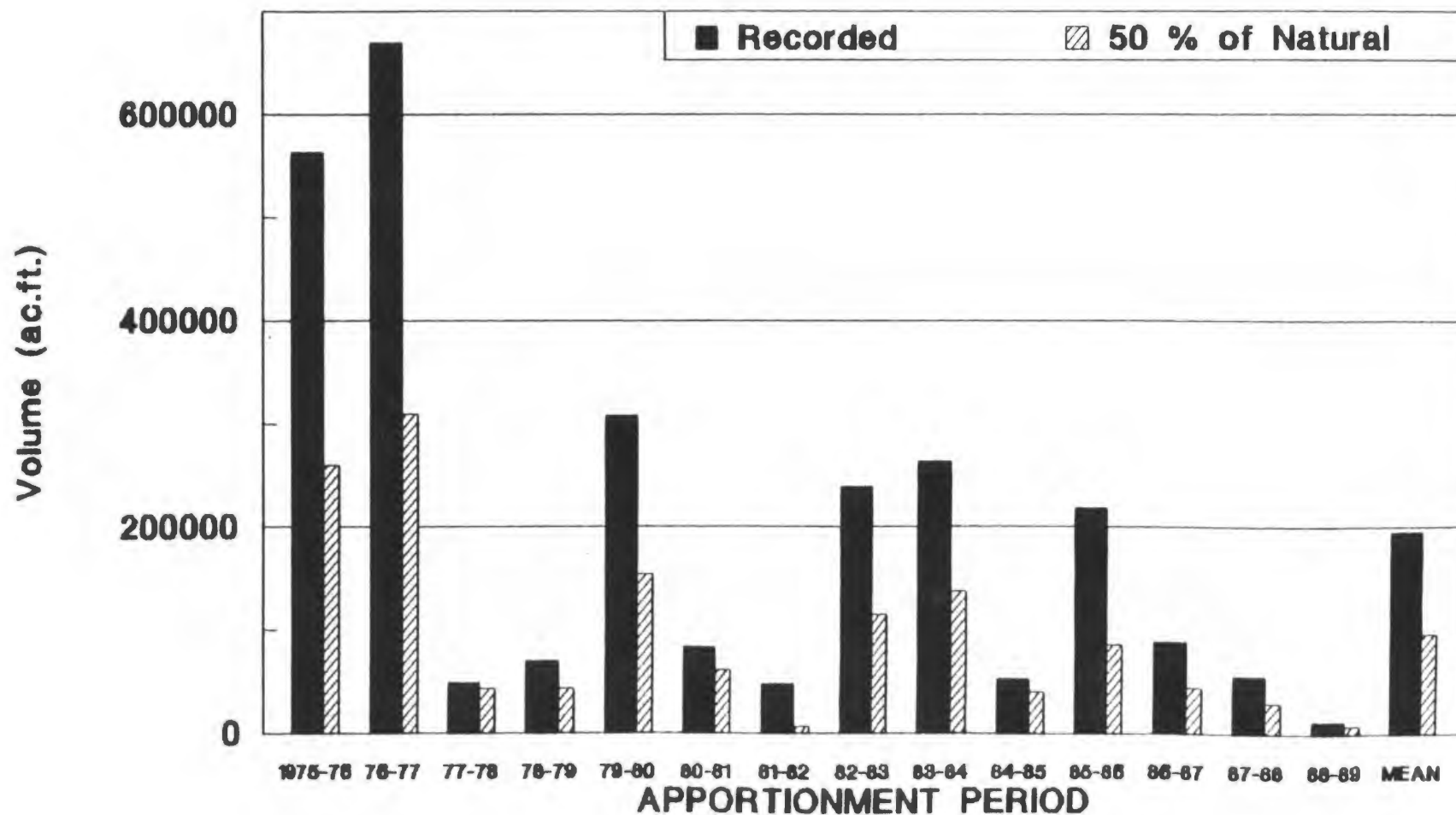
Flow volumes recorded at the Saskatchewan/Manitoba Boundary on the Qu'Appelle River for the 1975/76 to 1988/89 apportionment periods are plotted in Figure 3.3-3. Also plotted in Figure 3.3-3 is the 50 percent of natural flow volumes simulated with the modified model. This figure shows that the Province of Saskatchewan delivered in excess of the required 50 percent of natural flow, as specified in the 1969 Master Agreement on Apportionment, in every period simulated.

Table 3.3-1 summarizes the data plotted in Figures 3.3-1 to 3.3-3.

Licensed diversions in the Saskatchewan portion of the Qu'Appelle River Basin total 79900 dam³. The mean annual release at the Qu'Appelle Dam for the 1975 to 1988 period was 77100 dam³. Thus, because the diversion of water into the Qu'Appelle River Basin is close to the amount of uses within

QU'APPELLE RIVER NEAR WELBY

APPORTIONMENT PERIOD FLOW VOLUMES



Note: Apportionment Period is April 1 to March 31

TABLE 3.3-1

QU'APPELLE RIVER NEAR WELBY
APPORTIONMENT PERIOD VOLUMES

Apportionment Period*	Recorded (ac.ft.)	Simulated Natural		Increase In Simulated Natural		Portion of Natural Delivered To Manitoba	
		Original Model (ac.ft.)	Modified Model (ac.ft.)	(ac.ft.)	(%)	Original Model (%)	Modified Model (%)
1975-1976	563400	490958	521224	30266	6	115	108
1976-1977	669400	631733	620393	-11340	-2	106	108
1977-1978	48654	66700	86015	19315	29	73	57
1978-1979	69333	69799	85505	15706	23	99	81
1979-1980	309250	246656	305914	59258	24	125	101
1980-1981	82319	57420	122412	64992	113	143	67
1981-1982	48254	24106	13163	-10943	-45	200	367
1982-1983	239433	157535	227745	70210	45	152	105
1983-1984	265099	199486	274034	74548	37	133	97
1984-1985	52646	31766	80589	48823	154	166	65
1985-1986	218508	138077	170934	32857	24	158	128
1986-1987	87767	91382	87911	-3471	-4	96	100
1987-1988	55484	28104	60054	31950	114	197	92
1988-1989	12894	4127	15720	11593	281	312	82
Mean	194460	159846	190830	30983	57	148	111

* Note: Apportionment Period is April 1 to March 31

the basin, the volume of flow at the Saskatchewan/Manitoba Boundary would be expected to be close to the volume of flow which would occur under natural conditions. The mean 1975 to 1988 apportionment period simulated natural volume of flow at the Saskatchewan/Manitoba Boundary was 190830 acre-feet (235390 dam³). The mean recorded flow volume for the same time period was 194460 acre-feet (239870 dam³), 102 percent of natural.

3.4 Committee on Groundwater Review Comments

Because of the significant contribution of groundwater to the Qu'Appelle River System, and because of the complex relationship between groundwater and surface water, the SSARR sub-committee recommended that the COG review the groundwater section of this report.

Comments from the COG varied from "acceptable", to "highly questionable", to "high by perhaps as much as an order of magnitude". The wide range in comments confirms that true groundwater into the Qu'Appelle River System is not known.

The natural flow estimates arrived at in this study are dependent on the groundwater inflow estimate determined in this study. This groundwater inflow estimate was determined using the best information available at this time. The natural flow estimate could change in the event of improved groundwater inflow data.

CHAPTER IV

CONCLUSIONS

The original Qu'Appelle River Natural Flow Model contained a number of inaccuracies which often created difficulty for those using the model and produced simulation results of questionable accuracy. As a result of the modifications made to the natural flow model in this study, the modified Qu'Appelle River Natural Flow Model is easier to use and produces more realistic simulation results.

Table 4-1 lists the modifications made to the Qu'Appelle River Natural Flow Model and summarizes the effect each modification has on the simulated natural flow in the Qu'Appelle River at the Saskatchewan/Manitoba Boundary.

Over the period of 1975 to 1988, the modified model simulated apportionment period volumes for the natural flow in the Qu'Appelle River at the Saskatchewan/Manitoba Boundary which averaged 19 percent higher than those simulated by the original model. However, in spite of the increase in the natural flow estimate, Saskatchewan has always delivered more than the required 50 percent of natural flow.

TABLE 3.4-1

SUMMARY OF EFFECTS OF MODIFICATIONS
ON SIMULATED NATURAL FLOW IN THE QU'APPELLE RIVER
AT THE SASKATCHEWAN/MANITOBA BOUNDARY

Modification	Effect On Natural Flow Estimate	Degree Of Effect
1. Convert Qu'Appelle River Natural Flow Model to Microcomputer Format	Negligible	Negligible
2. Extend Last Mountain Lake Tables	No Effect	No Effect
3. Revise Last Mountain Lake Overflow Table (CF20)	Decrease	Minor
4. Revise Channel Routing Parameters	Negligible	Negligible
5. Revise Overbank Flow Reservoirs	Decrease	Minor
6. Revise Procedure for Simulating Reservoir Regulation	Increase	Major
7. Revise procedure for handling evaporation so that provision is made for lower evaporation under Natural Conditions	Increase	Major
8. Revise procedure for estimating local inflow	Increase	Minor
9. Incorporate effects of groundwater (Last Mountain Lake and Fishing Lakes)	Increase	Major
10. Correct Input Data Format	Increase	Minor

CHAPTER V
RECOMMENDATIONS

As a result of completing the Qu'Appelle River Natural Flow Model Modification Study, the following recommendations are made:

1. That the microcomputer of SSARR be used for running Qu'Appelle River Natural Flow simulations;
2. That the modified version of the Qu'Appelle River Natural Flow Model be used for estimating the natural flow in the Qu'Appelle River at the Saskatchewan/Manitoba Boundary;
3. That the Qu'Appelle River Natural Flow Model be left in Imperial Units as the time required to convert the model would not be cost-effective;
4. That a user's manual for the modified Qu'Appelle River SSARR Model be written to document the modified data input requirements;

REFERENCES

Hydrological Atlas of Canada, Environment Canada, 1978.

PFRA Hydrology Report No. 121, Gross Evaporation for the 30 year period 1951-80 in the Canadian Prairies November, 1989.

Streamflow Synthesis and Reservoir Regulation (SSARR) User Manual, U.S. Army Corps of Engineers, August, 1987.

Micro Computer Version of The SSARR Model, U.S. Army Corps of Engineers, August, 1987.

Qu'Appelle River at Saskatchewan/Manitoba Boundary, Natural Flow, PPWB, December, 1975.

Qu'Appelle River at Saskatchewan/Manitoba Boundary, Natural Flow, User Manual, PPWB, December, 1975.

Report of The Qu'Appelle Basin Study Board, Canada-Saskatchewan-Manitoba, 1972.

Fort Qu'Appelle Geology, Saskatchewan Research Council, 1977.

APPENDIX A

MEMORANDUM OF UNDERSTANDING

PRAIRIE PROVINCES WATER BOARD

COMMITTEE ON HYDROLOGY

MEMORANDUM OF UNDERSTANDING

Made in duplicate.

BETWEEN The Prairie Provinces Water Board (hereinafter referred to as "the Board").

AND Saskatchewan Water Corporation (hereinafter referred to as the "Participating Agency").

TITLE Qu'Appelle River SSARR Model Modification Study.

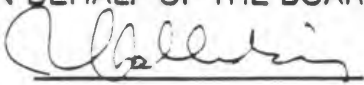
ARTICLES OF UNDERSTANDING

1. The Participating Agency will perform the services defined in the attached document "Schedule A - Work Required".
2. The Participating Agency will, upon completion of the tasks described in Schedule A, make the information available to the Board.
3. The Board will reimburse the Participating Agency for the work identified in Schedule A in accordance with allowable expenditures, as listed in the attached "Schedule B - Allowable Expenditures", up to a maximum of \$29,000.00.
4. The work shall be completed by March 31, 1990 or such other date as may be mutually agreed upon.
5. The performance of the parties to the Memorandum of Understanding is subject to the conditions contained in the attached document "Schedule C - General Conditions".
6. Amendments to the Memorandum of Understanding, including changes of the completion date, may be accomplished by an exchange of letters by the signatories.

AUTHORIZATION

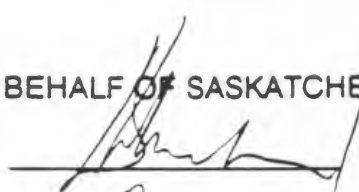
As recommended by the Chairman of the Board, and in compliance with the Board's rules and procedures, the Board and the Participating Agency, therefore, enter into this Memorandum of Understanding.

SIGNED ON BEHALF OF THE BOARD:

By 

on 89-04-24

SIGNED ON BEHALF OF SASKATCHEWAN WATER CORPORATION:

By 

on 89.04.26

SCHEDULE A - WORK REQUIRED

1. Implement the existing Qu'Appelle River Basin SSARR model on a micro-computer. Ensure the current Qu'Appelle River basin micro-computer SSARR model is operational on the micro-computer.
2. Extend stage-area and stage-capacity curves of Last Mountain Lake to cover the entire range of lake levels and incorporate these curves into the SSARR model.
3. Revise channel routing parameters for post conveyance conditions. Determine the reaches where modifications have been carried out, what was done and when it was completed. Enter these revised routing parameters into the SSARR model.
4. Determine reservoir regulation operational procedures (past and present) for each reservoir in the Qu'Appelle River system and incorporate into SSARR model using reservoir regulation cards.
5. Incorporate evaporation cards into the model taking into consideration increased lake areas due to regulation of lake elevations.
6. Revise procedure for computing ungauged lateral flow. Determine local drainage areas (gross and effective) of each river reach in the Qu'Appelle River system. Estimate natural inflow using drainage area ratios and natural recorded flows at representative WSC gauging stations. Implement this method of estimating local inflows into SSARR model.
7. Evaluate the effects of groundwater flow on Last Mountain Lake and the Fishing Lakes, and, if applicable, incorporate a groundwater component into the model.
8. Re-run the revised model, summarize the results and compare new results to the current model results.
9. Submit interim reports and financial claims quarterly during the study period and submit a draft report by December 1, 1989.
10. Prepare 40 copies of a report on the work done, results obtained and any recommendations in a form satisfactory to the Executive Director by March 31, 1990. All reports and the original documents from which the reports are produced will become the property of the Board.

SCHEDULE B - ALLOWABLE EXPENDITURES

For the purposes of the Board, allowable expenditures comprise those costs associated with carrying out the studies and investigations pursuant to Schedule A of this Memorandum of Understanding. They shall include:

1. Salaries of employees computed on a minimum half-day basis.
2. Transportation and living expenses of such employees while away from their normal headquarters.
3. Rental charges for equipment owned by their parties, while being used for the provision of services, including operators wages.
4. All out-of-pocket operating, maintenance, and transportation expenses for equipment owned by the Participating Agency for the provision of services.
5. Cost of materials, expenses and services including computer time incurred for the provision of work approved by the Board Secretariat.
6. Costs associated with contracts for services.
7. Excluded are indirect overhead costs such as office space, depreciation of furniture, supervisory costs and other items that would represent a normal cost item for the contractor in operating the agencies regular work program.

SCHEDULE C - GENERAL CONDITIONS

Interpretation

1. In this Memorandum of Understanding:
 - (a) "the Board" - means the Prairie Provinces Water Board as established under Schedule C of the Agreement dated the thirtieth day of October, 1969, between the Governments of Canada, Manitoba, Saskatchewan, and Alberta.
 - (b) "Executive Director" - means the senior employee of the Board; subject to the Board's directions, responsible for the technical and administrative activities and the day-to-day management of the Board.
 - (c) "Standards" - standards established between the Board Secretariat and/or the Committee on Hydrology.
 - (d) "Schedule" - means the order and timing of studies and investigations as set out in Schedule A.
 - (e) "Participating Agency" - means the agency which will undertake the studies and investigations.
2. The report will be prepared in accordance with the Standards.
3. Any changes to the scope and/or schedule of the work set out in Schedule A deemed to be significant by the Executive Director, must receive prior approval of the Committee on Hydrology and be duly authorized.
4. All work will be conducted in accordance with the schedule. Provided, however, that if in the opinion of the Executive Director, it is considered in the mutual interests of the Participating Agency and the Board, he may revise the Schedule following consultation with the Participating Agency.
5. In the case where the Board or the Participating Agency anticipates serious delays in the completion of the services or of a part thereof within the time set for its completion, the Board may take all or any part of Schedule A out of the hands of the Participating Agency upon receipt of or after giving 30 days written notice.
6. Where the Memorandum of Understanding, or any portion thereof has been terminated, for any reasons, the Board is not obligated to pay the Participating Agency for any expenses after the written notice of termination has been received by the Participating Agency other than those expenses related to the winding up of activities or any part thereof.

7. The allowable expenditures stipulated in Article 3 of this Memorandum of Understanding shall not be exceeded by the Participating Agency without the prior appropriate amendment of the memorandum.
8. This Memorandum of Understanding may be amended by exchange of letters between the Board and the Participating Agency.
9. The Participating Agency shall maintain records, consistent with accounting practices, of all expenditures made pursuant to this Memorandum of Understanding supported by proper documents and vouchers. Such records, documents and vouchers shall be made available to the Board for audit upon request and the Participating Agency shall furnish any and all information in relation thereto.
10. The Participating Agency agrees to pay all debts and liabilities incurred in the performance of services under this Memorandum of Understanding.
11. Claiming Procedures
 - a. The Participating Agency will submit claims at the end of each fiscal year quarter for work completed under this Memorandum of Understanding during the previous three-month period.
 - b. Claims for work done in the last quarter of each fiscal year will be submitted by the Participating Agency not later than March 15th of that year and will include an estimate of all expenditures to be incurred to the end of the current fiscal year.
 - c. Adjustments to the claim for the last fiscal year quarter, to account for discrepancies between estimated and actual expenditures, will be reflected in the claim for the first quarter of the following fiscal year.
 - d. Payment of claims for any fiscal year is subject to the availability of funds for that particular year.
 - e. All claims by the Participating Agency for work done under this Memorandum of Understanding will be submitted to the Executive Director of the Board, Regina, Saskatchewan.

APPENDIX B

ORIGINAL QU'APPELLE SSARR MODEL CONFIGURATION CHARTS

Source: Natural Flow, Qu'Appelle River at
Saskatchewan/Manitoba Boundary
Environment Canada, December, 1975

TECHNICAL REPORT TO THE
PPWB COMMITTEE ON HYDROLOGY

NATURAL FLOW

QU'APPELLE RIVER

AT SASKATCHEWAN MANITOBA BOUNDARY

PREPARED BY:

DEPARTMENT OF ENVIRONMENT
WATER SURVEY OF CANADA
ATMOSPHERIC ENVIRONMENT SERVICE

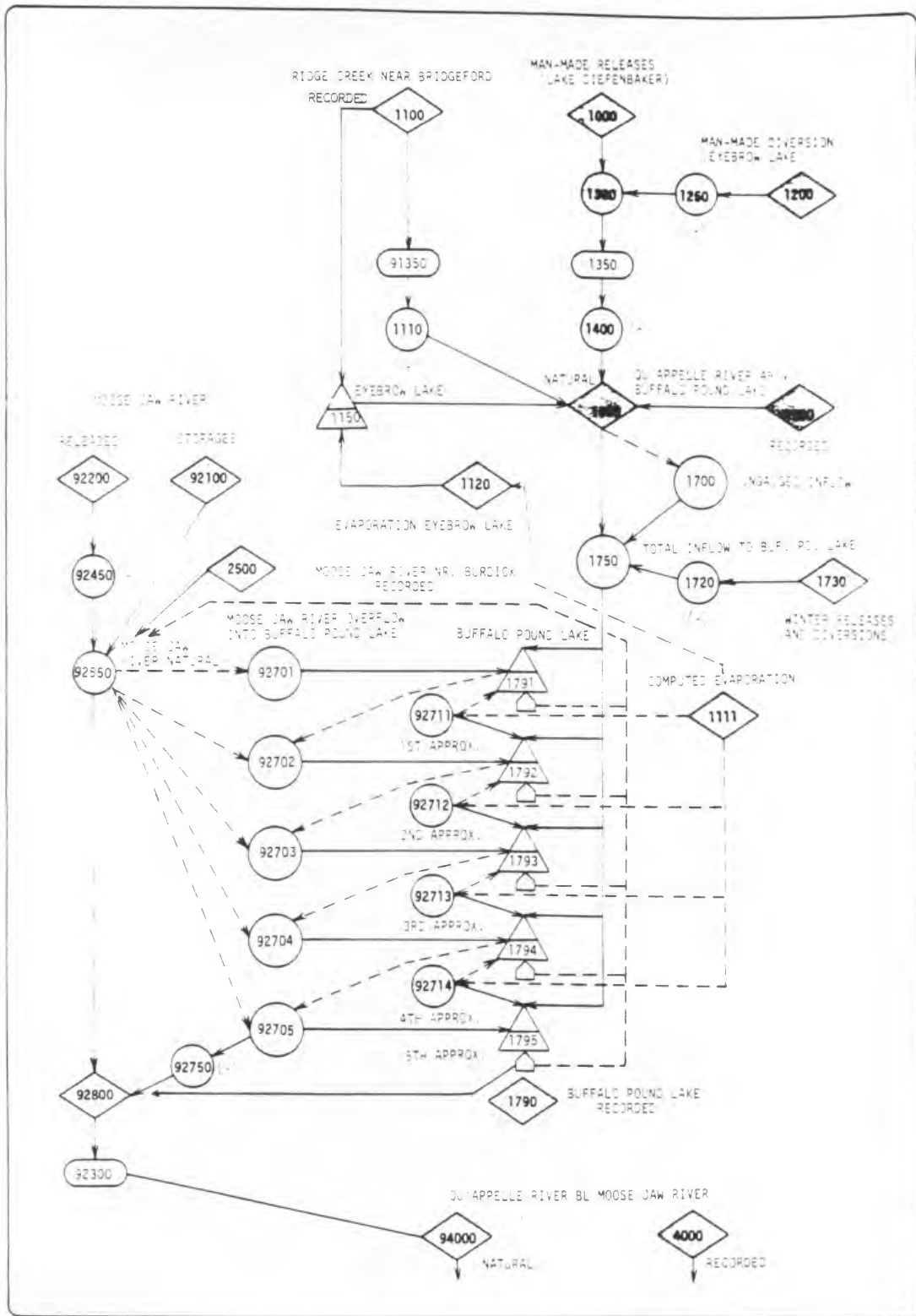


Figure 13: SSARR River System Chart – Lake Diefenbaker to below Moose Jaw River.

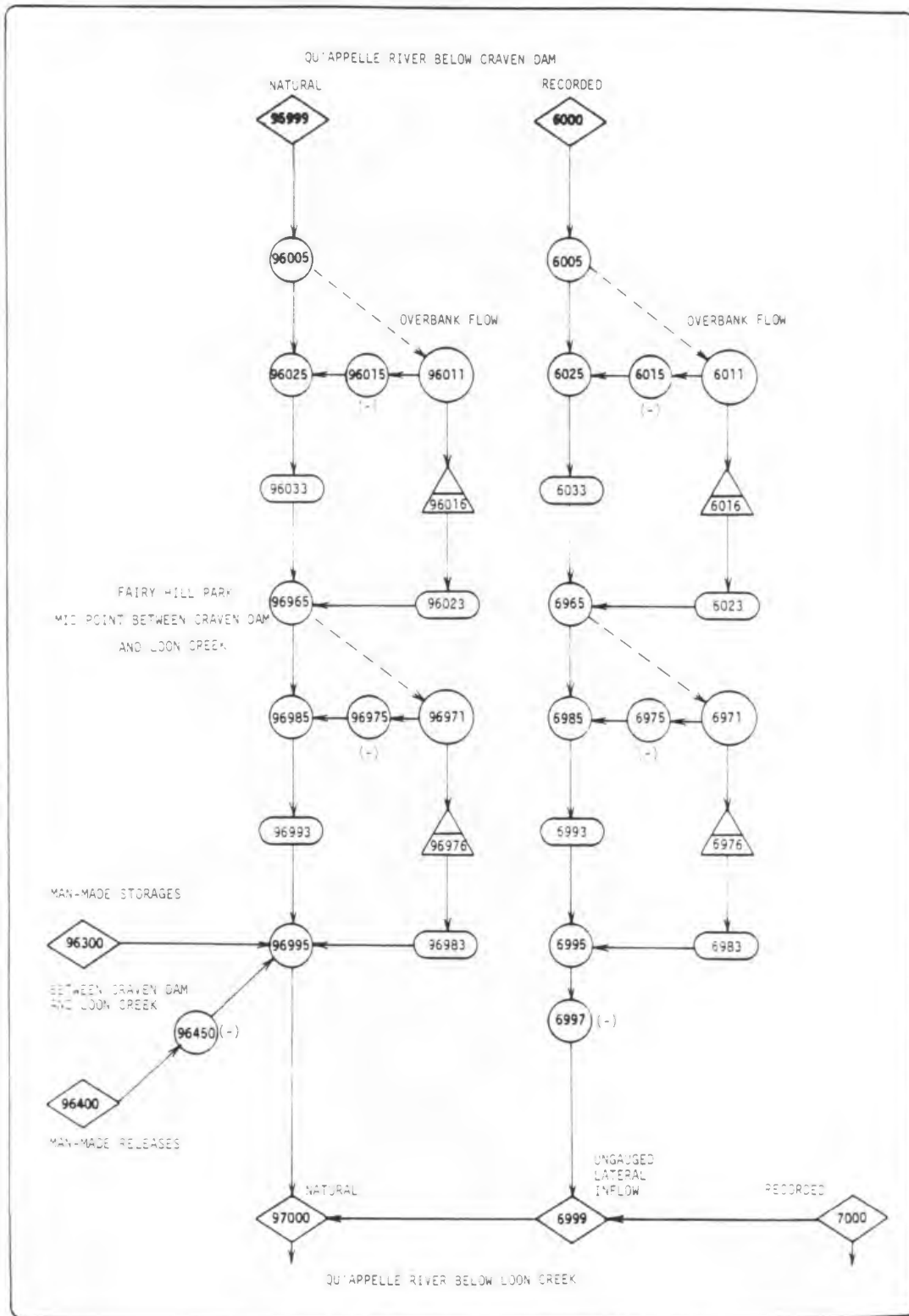


Figure 15: SSARR River System Chart — below Craven Dam to below Loon Creek.

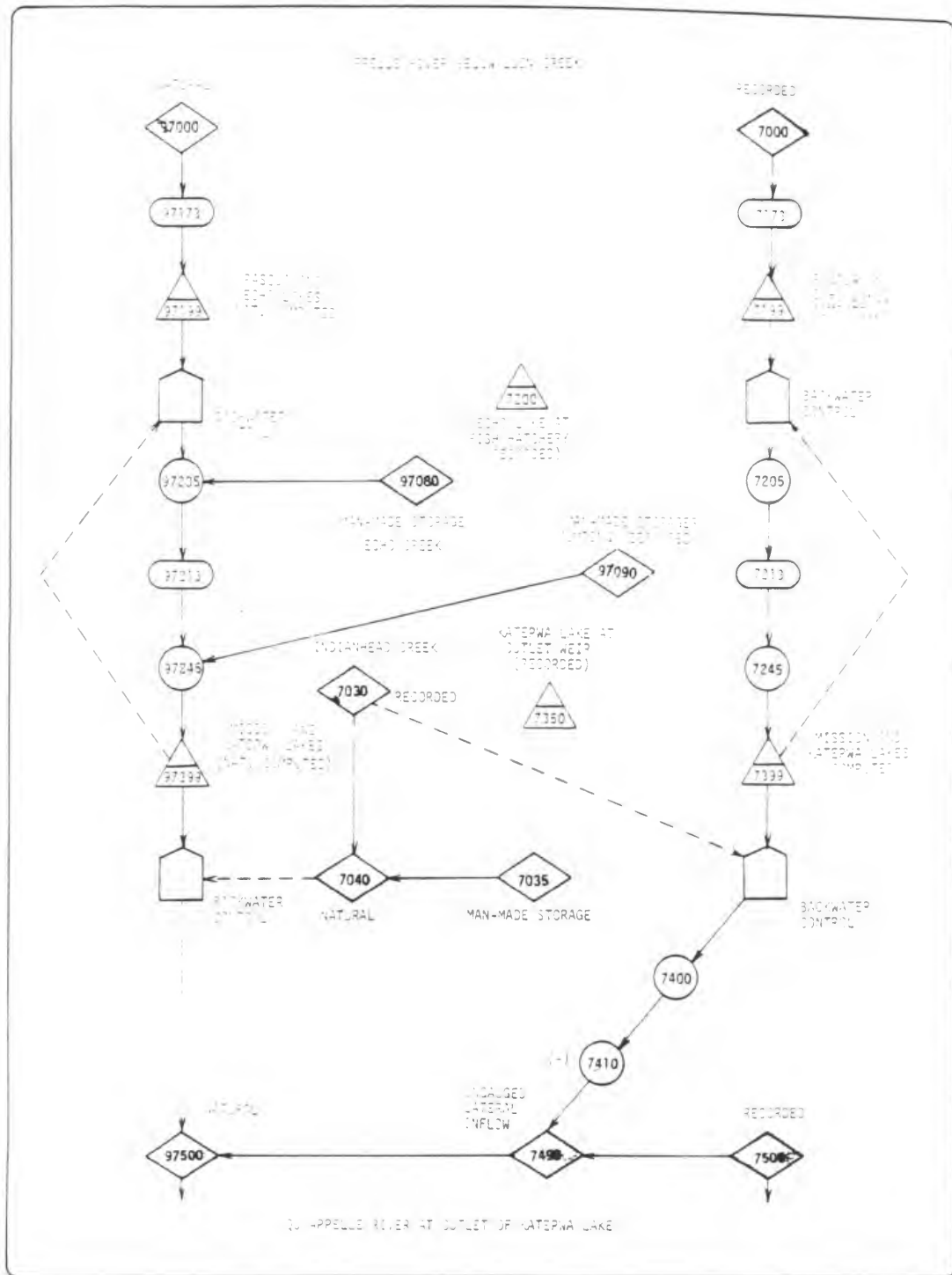


Figure 16: SSARR River System Chart — below Loon Creek to Outlet of Katepwa Lake.

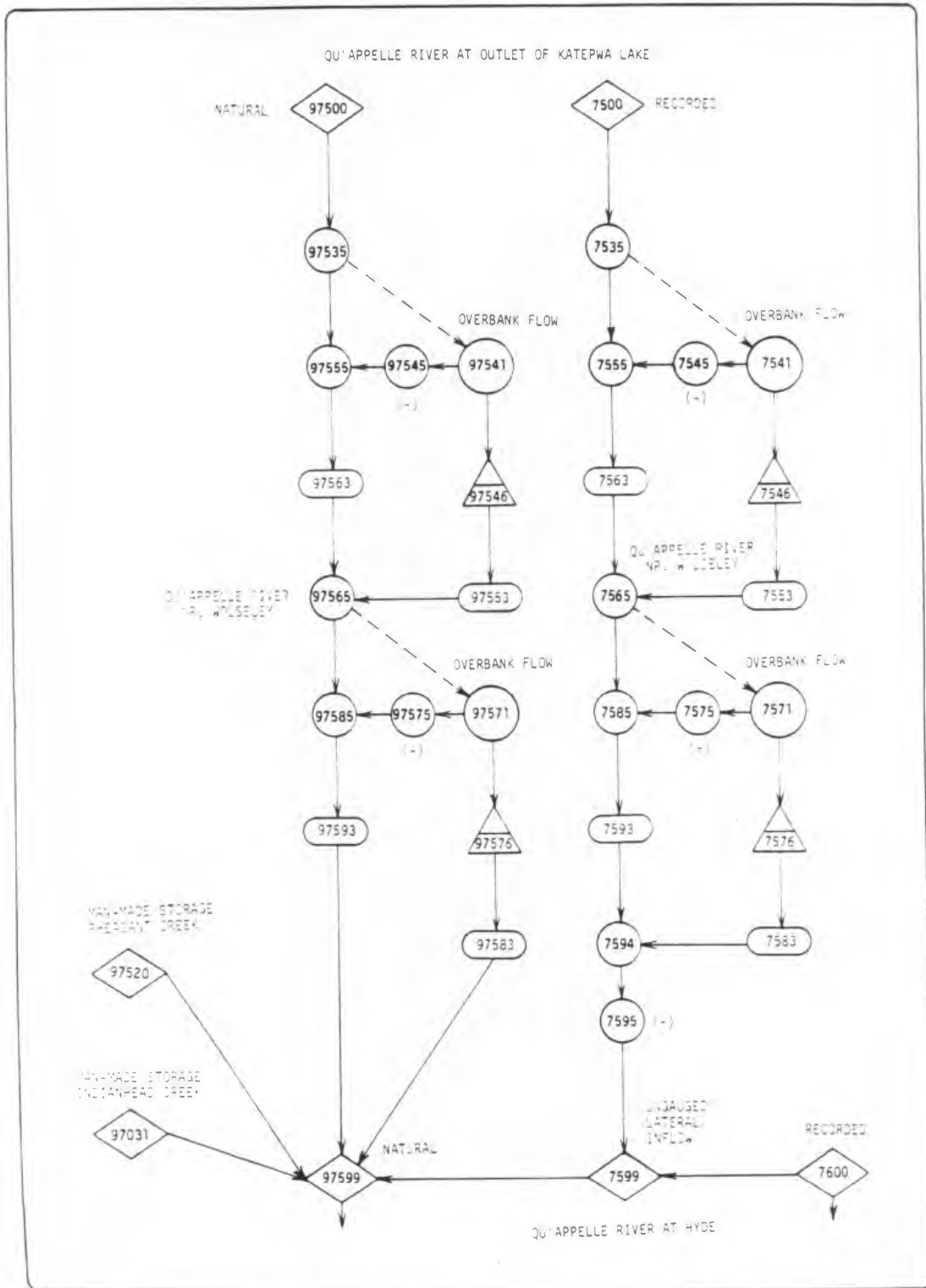


Figure 17: SSARR River System Chart – Outlet of Katepwa Lake to Hyde.

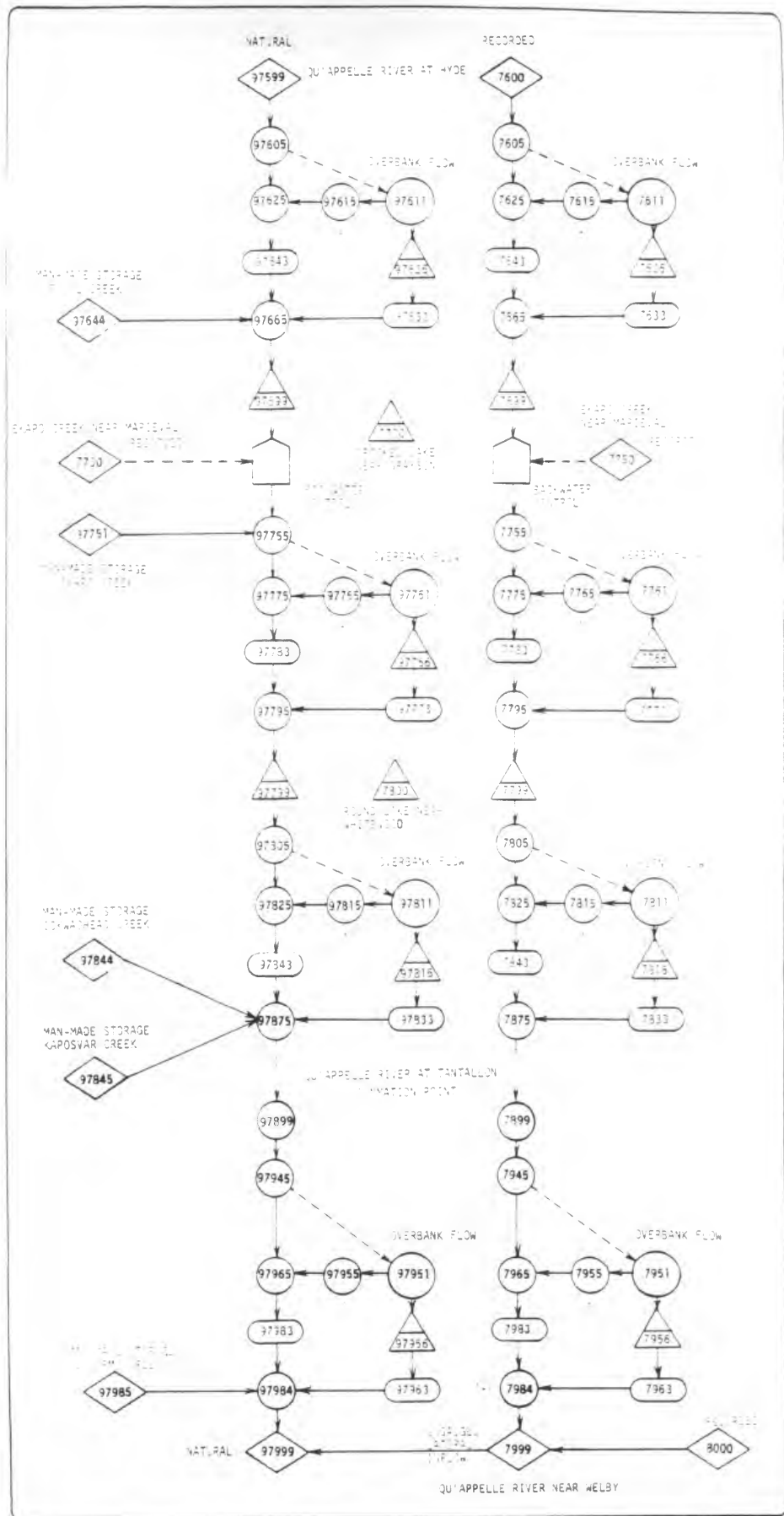


Figure 18: SSARR River System Chart -- Hyde to Welby.

APPENDIX C

INSTRUCTIONS FOR RUNNING MODIFIED QU'APPELLE
SSARR NATURAL FLOW MODEL ON MICRO COMPUTER

C:\SSARR>FINIT

THIS PROGRAM WILL EMPTY-OUT FILE\IODC
IF YOU ENTER A 'Y':

Y

FILE\IODC INITIALIZED AS EMPTY.
Stop - Program terminated.

C:\SSARR>J

C:\SSARR>jsrexe con con
1234567890123456789012345678901 ENTER SSARR CARD 60 234567890123456789
INFILE Q1288.FIN

INFILE Q1288.FIN

JOB 0000280389 0 0 24011 1 ELBOW TO WELBY MARCH 1 TO JUL 31 1988

CT 30 2 0 0 50 0 150 50

:

6S 7668 1200112881 012031121 0

END OF OUTPUT

Stop - Program terminated.

C:\SSARR>IA

INTERACTIVE SSARR MONITOR. ENTER ? FOR INSTRUCTIONS

RIVER Q12

RIVER Q12

MODEL Q12 RUNNING RIVER MODEL FOR 153 PERIODS.

Stop - Program terminated.

FINAL FILE OF TIME RECORDS IS ON UNIT 22.

Stop - Program terminated.

Enter SSARR main command: "J", "Z", "MAINT", "PR", "PL", or "IA".

C:\SSARR>PRDCE CON CON

PR, X, AND SR CARD PROCESSOR:

PR AND PT CARDS PRODUCE FORMATTED PRINTOUT FROM RIVER MODEL.

X CARDS PRODUCE TELETYPE REPORT.

SR, SQ, AQ, SE, AE, ZSQ, ZAQ, ZSE, ZAE CARDS SAVE RIVER MODEL
MODEL COMPUTED FLOWS AND ELEVATIONS INTO SSARR FILE.

1234567890123456789012345678901 ENTER SSARR CARD 60 234567890123456789

INFILE Q12.PCD

INFILE Q12.PCD

PR 8	17902	17952	940001	40001				11
PRC8	860002	958042	959991	60001				11
PR 8	72002	71992	1971992					11

UNIT 8? Q1288.FOP

PRC8	73502	73992	1973992					11
PRC8	75001	74001	975001					11
PR 8	74991	77002	76992	1976992				11
PRC8	78002	77992	1977992					11
PR 8	80001	79841	979991	79991				11
PR 8	S10004	S40004	S940004	S959994	S60004			11
PRC8	S75004	S74004	S74994	S975004				11
PR 8	S80004	S79844	S79994	S979994				11
PRC8	SEVPS4	SEVFN4	S50004	S949004				11
PR 8	11503	17953	958043	953351	9599911203107	1200108	240011	
PR 8	1971993	1973993	1976993	1977991	9799911203107	1200108	240011	
PR 8	72006	71996	73506	73996	1203107	1200108	240011	
PR 8	77006	76996	78006	77996	1203107	1200108	240011	

BY

END OF INPUT

Stop - Program terminated.

C:\SSARR>

APPENDIX D

MODIFIED QU'APPELLE RIVER SSARR NATURAL FLOW MODEL

SAMPLE INPUT FILES

Spring and Summer Model: Page D-1

Fall and Winter Model: Page D-26

CJ	1792	200	166700	-1	200	166940	300
CJ	1792	200	167250	700	200	167500	1200
CJ	1792	200	167720	2000	200	169200	16000
CJ	1792	300	166800	-1	300	167030	300
CJ	1792	300	167300	700	300	167550	1200
CJ	1792	300	167760	2000	300	169300	16000
CJ	1792	1000	167500	-1	1000	167670	300
CJ	1792	1000	167770	700	1000	167880	1200
CJ	1792	1000	168000	2000	1000	169400	16000
CJ	1792	3000	167500	-1	3000	167670	300
CJ	1792	3000	167770	700	3000	167880	1200
CJ	1792	10000	168000	-1	10000	169400	15000
CJ	1792	-999999					
CL01	1793	1					
CL02	1793	1					
C110	1793	1653					
C111	1793	1655					
C112	1793	1657					
C113	1793	1659					
C114	1793	1661					
C115	1793	1665					
C116	1793	1675					
C117	1793	1685					
CJ	1793	-1	1653	0	-1	1654	16000
CJ	1793	0	166300	-1	0	166750	300
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CJ	1793	200	167250	700	200	167500	1200
CJ	1793	200	167720	2000	200	169200	16000
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CJ	1793	300	167300	700	300	167550	1200
CJ	1793	300	167760	2000	300	169300	16000
CJ	1793	1000	167500	-1	1000	167670	300
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CL01	1794	1					
CL02	1794	1					
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C116	1794	1675					
C117	1794	1685					
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CJ	1794	100	167200	700	100	167450	1200
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CJ	1794	-999999					
CL01	1795	1					
CL02	1795	1					
C110	1795	1653					
C111	1795	1655					
C112	1795	1657					
C113	1795	1659					
C114	1795	1661					
C115	1795	1665					
C116	1795	1675					
C117	1795	1685					
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CJ	1795	300	167760	2000	300	169300	16000
CJ	1795	1000	167500	-1	1000	167670	300
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CJ	1795	1000	168000	2000	1000	169400	16000
CJ	1795	-999999					
CC01	92711	0	112				
CC02	92711	1111					
CC01	92702	0	121				
CC02	92702	1791					
CC01	92712	0	112				
CC02	92712	1111					
CC01	92703	0	121				
CC02	92703	1792					
CC01	92713	0	112				
CC02	92713	1111					
CC01	92704	0	121				

C2	7399	800	157015	-10	800	157107	1200
C2	7399	1000	157035	-10	1000	157132	1200
C2	7399	1500	157091	-10	1500	157203	1200
C2	7399	2000	157158	-10	2000	157264	1200
C2	7399	2500	157225	-10	2500	157318	1200
C2	7399	3000	157284	-10	3000	157370	2000
C2	7399	10000	157850	-10	10000	157900	3000
C2	7399	-999999					
CC01	47199	1				RIPARIAN RELEASE OUT OF ECHO LAKE	
CC01	47198	1				SIGN CHANGE	
CC01	47198	1					
CC01	47399	1				RIPARIAN RELEASE OUT OF KATEPWA LAKE	
CC01	47398	1				SIGN CHANGE	
CC01	47398	1					
CC01	7400	0				KATEPWA LAKE AT OUTLET WEIR - SUMMATION POINT	
CC01	7410	0				SIGN REVERSAL	
CC02	7410	1					
CC01	7499	1				QUAPPELLE AT OUTLET KATEPWA WEIR UNGAUGED LAT INFL	
CC01	37499	1				GROUNDWATER AND UNGAUGED INFLOW ADJUSTMENT	
CC02	37499					7499 0500	
CC01	7500	0				QUAPPELLE RIVER AT OUTLET OF KATEPWA LAKE REC	
CC01	97500	0				QUAPPELLE RIVER OF OUTLET OF KAT L	
CC01	97535	0				QUAPPELLE BEL KATEPWA LAKE SUMMED - SUMMED	
CC01	97541	0				OVERBANK FLOW COMPUTED USING TABLE 60	
CC02	97541					97535 601000	
CC01	97545	0				SIGN REVERSAL	
CC02	97545	1					
CC01	97555	0				QUAPPLE RIVER WITHOUT OVERBANK FLOW	
CRO1	97563	0				QUAPPELLE - ROUTED	
CRO2	97563	8	350	6460			
CLO1	97546	0				LAKE CONTAINING OVERBANK FLOW	
CLO2	97546					600 100	
C110	97546	100	1	10	200	1	3500
C111	97546	500	1000	15000	600	10000	20000
C113	97546	999999	999999	999999	999999	999999	
CRO1	97553	0				OVERBANKFLOW ROUTED	
CRO2	97553	8	353	5599			
CC01	97565					QUAPPELLE AND LOCAL - SUMMED	
CC01	97571	0				OVERBANK FLOW COMPUTED USING TABLE 60	
CC02	97571					97565 601000	
CC01	97575	0				SIGN REVERSAL	
CC02	97575	1					
CC01	97585	0				QUAPPLE RIVER WITHOUT OVERBANK FLOW	
CRO1	97593	0				QUAPPELLE - ROUTED	
CRO2	97593	8	353	5599			
CLO1	97576	0				RESERVOIR CONTAINING OVERBANK FLOW	
CLO2	97576					600 100	
C110	97576	100	1	10	200	1	3000
C111	97576	500	1000	15000	600	10000	20000
C113	97576	999999	999999	999999	999999	999999	
CRO1	97583	0				OVERBANK FLOW ROUTED	
CRO2	97583	8	353	5599			
CC01	97031	0				MAN MADE STORAGE ON INDIANHEAD CREEK	
CC01	97520	0				MAN MADE STORAGE ON PHEASANT CREEK	
CC01	7535	0				QUAPPELLE BEL KATEPWA LAKE SUMMED - SUMMED	
CC01	7541	0				OVERBANK FLOW B. KAT. LK (CT 34 PRE- 1980)	
CC02	7541					7535 341000 (CT 37 POST 1980)	
CC01	7545	0				SIGN REVERSAL	
CC02	7545	1					
CC01	7555	0				QUAPPELLE RIVER WITHOUT OVERBANK FLOW	
CRO1	7563	0				QUAPPELLE - ROUTED	
CRO2	7563	8	350	6460			
CLO1	7546	0				LAKE CONTAINING OVERBANK FLOW	
CLO2	7546					600 100	
C110	7546	100	1	10	200	1	3000
C111	7546	500	1000	15000	600	10000	20000
C113	7546	999999	999999	999999	999999	999999	
CRO1	7553	0				OVERBANK FLOW ROUTED	
CRO2	7553	8	353	5599			
CC01	37565	1	111			UNGAUGED LOCAL INFLOW	
CC02	37565	7040				RETPER 3751000 INTO QUAPPELLE @ HYDE	
CC01	7565	0				QUAPPELLE AND LOCAL - SUMMED	
CC01	7571	0				OVERBANK FLOW COMPUTED USING TABLE 60	
CC02	7571					7565 601000	
CC01	7575	0				SIGN REVERSAL	
CC02	7575	1					
CC01	7585	0				QUAPPELLE RIVER WITHOUT OVERBANK FLOW	
CRO1	7593	0				QUAPPELLE - ROUTED	
CRO2	7593	8	353	5599			
CLO1	7576	0				RESERVOIR CONTAINING OVERBANK FLOW	
CLO2	7576					600 100	
C110	7576	100	1	10	200	1	3000
C111	7576	500	1000	15000	600	10000	20000
C113	7576	999999	999999	999999	999999	999999	
CRO1	7583	0				OVERBANK FLOW ROUTED	
CRO2	7583	8	353	5599			
CC01	7594	0				SUMMATION POINT	
CC01	7595	0				SIGN REVERSAL	
CC02	7595	1					
CC01	7599	1				QUAPPELLE RIVER AT HYDE - UNGAUGED LAT INFLOW	
CC01	7600	1				QUAPPELLE RIVER AT HYDE - RECORDED	
CC01	97599	0				QUAPPELLE RIVER AT HYDE - NATURAL FLOW	
CC01	97605	0				QUAPPELLE RIVER BELOW HYDE - SUMMED	
CC01	97611	0				OVERBANK FLOW COMPUTED USING TABLE 60	
CC02	97611					97605 601000	
CC01	97615	0				SIGN REVERSAL	
CC02	97615	1					
CC01	97625	0				QUAPPELLE RIVER WITHOUT OVERBANK FLOW	
CLO1	97626	0				DEAD STORAGE RESERVOIR	
CLO2	97626					600 100	
C110	97626	100	1	10	200	1	1500
C111	97626	500	1000	10000	600	10000	15000
C113	97626	999999	999999	999999	999999	999999	
CRO1	97643	0				QUAPPEL RIVER BELOW HYDE - ROUTED	
CRO2	97643	3	386	6810			
CRO1	97633	0				OVERBANK FLOW - ROUTED	
CRO2	97633	3	386	6810			
CC01	97644	0				MAN MADE STORAGE PEARL CREEK	

P 74	93700	94405				
P 75	93800	93900				
P 76	93900	94405				
P 77	94220	95305				
P 78	34122	94405	4405			
P 79	94405	94900				
P 80	86000					
P 81	4000	4205	S4000			
P 82	4205	4235				
P 83	4211	4225	4236			
P 84	4236	4253				
P 86	4253	4405				
P 87	4225	4235				
P 88	4235	4263				
P 89	4263	4405				
P 90	4405	4410				
P 91	4410	4900				
P 92	5000	4900	S5000			
P 93	4900	94900				
P 94	5360	95305				
P 95	5350	95305				
P 96	94900	95305	S94900			
P 97	95305	95323				
P 98	95323	95335				
P 00	95335	95365				
P 01	95200	95210				
P 02	2222					
P 03	5111	5112	SEVPS			
P 04	5112	95353				
P 05	95353	95353				
P 06	95210	95353				
P 07	95210	95549	95801	95802	95803	95804
P 08	95336	95549				
P 09	95549					
P 10	95701	95801				
P 11	95711	95801				
P 12	95801					
P 13	95702	95802				
P 14	95712	95802				
P 15	95802					
P 16	95703	95803				
P 17	95713	95803				
P 18	95803					
P 19	95714	95804	S95714	SEVPN		
P 20	95714	95804	95345			
P 21	95704					
P 22	95804					
P 23	95345	95365				
P 24	5342	CARD REMOVED				
P 25	95365	95999				
P 26	6000	6000	6005			
P 26	95999	96005	S95999			
P 27	7035	CARD REMOVED				
P 27	7176					
P 28	7668					
P 29	7200					
P 30	7350					
P 31	7700					
P 32	7800					
P 33	27199					
P 34	27699					
P 35	27799					
P 36	7030	7040	7535			
P 37	97031	7040				
P 38	7040	97535				
P 39	7750	97750	7755			
P 40	97751	97750				
P 41	97750	97755				
P 42	96005	96025				
P 43	96011	96015	96016			
P 44	96016	96015				
P 45	96023	96965				
P 46	96015	96025				
P 47	96025	96033				
P 48	96033	96965				
P 49	96965	96985				
P 50	96971	96975	96976			
P 51	96976	96983				
P 52	96983	96995				
P 53	96975	96985				
P 54	96985	96993				
P 55	96993	96995				
P 56	96300	96995				
P 57	96400	96430				
P 58	96430	96995				
P 59	96995	97000				
P 60	6005	6025	6016			
P 61	6011	6015				
P 62	6016	6023				
P 63	6023	6965				
P 64	6015	6025				
P 65	6025	6033				
P 66	6033	6965				
P 67	6965	6985				
P 68	6971	6976	6975			
P 69	6976	6983				
P 70	6983	6995				
P 71	6975	6985				
P 72	6985	6993				
P 73	6993	6995				
P 74	6995	6997				
P 75	6997	6999				
P 76	7000	6999	7005			
P 77	7011	7015	7016			
P 78	7016	7183				
P 79	7183	7025				
P 80	7183	7005				
P 81	7015	7005				
P 82	7005	7173				

P 83	7173	7025		
P 84	37199	97205	7025	
P 85	7025	7199		
P 86	7174	7199	S7174	SEVPS
P 87	37399	97245	7245	
P 88	7199	7205		
P 89	7205	7213		
P 90	7213	7245		
P 91	7245	7247	7399	SEVPS
P 92	7245	7399		
P 93	7399	7400		
P 94	7400	7410	S7400	
P 95	7410	7499		
P 96	7500	7499	7535	S7500
P 97	7499	37499	S7499	
P 98	37499	97025	97245	
P 99	6999	97000		
P 00	97000	97005		
P 02	97011	97015	97016	
P 03	97016	97183		
P 04	97183	97025		
P 05	97015	97005		
P 06	97005	97173		
P 07	97173	97025		
P 08	97025	97199	197199	297199
P 09	97199	97204		
P 10	97174	197199	S97174	
P 11	197199	97204		
P 12	297174	297199	SEVFN	
P 13	297199	97205		
P 14	97080	97205		
P 15	97205	97213		
P 16	97213	97245		
P 17	97090	97245		
P 18	97245	97399	197399	297399
P 19	97399	97244		
P 20	97246	197399		
P 21	197399	97244		
P 22	297246	297399	SEVFN	
P 23	297399	97500	S97500	
P 24	97500	97535		
P 24	97535	97555		
P 25	97541	97545	97546	
P 26	97545	97555		
P 27	97555	97563		
P 28	97563	97565		
P 29	37565	97565	7565	
P 30	97546	97553		
P 31	97553	97563		
P 32	97565	97582		
P 33	97571	97575	97576	
P 34	97575	97585		
P 35	97585	97593		
P 36	97576	97583		
P 37	97593	97599		
P 38	97593	97599		
P 39	97583	97599		
P 40	97520	97599		
P 42	7535	7555		
P 43	7541	7545	7546	
P 45	7545	7555		
P 46	7555	7563		
P 47	7563	7565		
P 48	7546	7553		
P 50	7553	7565		
P 51	7565	7585		
P 53	7571	7575	7576	
P 54	7575	7585		
P 55	7585	7593		
P 56	7593	7594		
P 57	7576	7583		
P 58	7583	7594		
P 59	7594	7595		
P 60	7595	7599		
P 61	7600	7599	7605	
P 62	7599	97599		
P 63	7605	7625		
P 64	7611	7615	7626	
P 66	7615	7625		
P 67	7625	7643		
P 68	7626	7633		
P 69	7633	7665		
P 70	7643	7665		
P 71	7666	7667	7699	SEVPS
P 72	37699	7665	97665	
P 73	7665	7699		
P 74	7699	7755		
P 75	7755	7775		
P 76	7761	7765	7766	
P 77	7765	7773		
P 78	7773	7783		
P 79	7783	7795		
P 80	7766	7773		
P 81	7773	7795		
P 82	7796	7797	7799	SEVPS
P 83	37799	7795	97795	
P 84	7795	7799		
P 85	7799	7805		
P 86	7985	7984	97986	
P 87	97985	97986		
P 88	97986	97984		
P 89	37899	7899	97899	
P 90	7805	7825		
P 91	7811	7815	7816	
P 92	7815	7825		
P 93	7816	7833		
P 94	7825	7843		
P 95	7843	7845		
P 96	7845	7875		
P 97	7833	7875		
P 98				

6D	9703111200108861	0	
6D	9708011200103861	0	
6D	9708021200203861	0	4
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6D	9751121200305861	0	6
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6D	97845211200203861	0	30
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6D	97845211200305861	0	28
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6D	9798521200305861	0	83
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6D	9798511200108861	0	

ECHO LAKE OUTLET ELEVATION

6S	27199	1200103862	15695012020032	156950
6S	27199	1202103862	15712312015062	157123
6S	27199	1201606862	15715212001082	157152

CROOKED LAKE OUTLET ELEVATION

6S	27699	1200103862	14794012006042	147940
6S	27699	1200704862	14814612029052	148146
6S	27699	1203005862	14819512002072	148195
6S	27699	1200307862	14819012001082	148150

ROUND LAKE OUTLET ELEVATION

6S	27799	1200103862	14460012002042	144600
6S	27799	1200304862	14509512029052	145090
6S	27799	1203005862	14514412017072	145144
6S	27799	1201807862	14510012001082	145100

ECHO LAKE RELEASE

6S	47199	1200103861	012001081	0
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KATEPWA LAKE RELEASE

6S	47399	1200103861	1012022031	10
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RECORDED DATA

6D	25008120	1	3861	211	236	215	172	39	38	50	126
6D	25008120	9	3861	197	231	309	308	302	291	245	212
6D	25008120	17	3861	128	192	113	145	117	88	131	124
6D	2500712025		3861	134	123	110	98	73	76	67	-1111
6D	25008120	1	4861	59	38	67	64	65	69	74	69
6D	25008120	9	4861	71	52	40	17	21	39	49	20
6D	25008120	17	4861	73	60	40	30	24	20	21	20
6D	2500612025		4861	20	19	18	17	16	14	14	-39235
6D	25008120	1	5861	8	5	5	5	32	76	43	-1111
6D	25008120	9	5861	213	253	314	417	480	530	590	629
6D	25008120	17	5861	611	565	466	351	265	209	182	139
6D	2500712025		5861	108	76	74	72	40	8	1	-1111
6D	25008120	1	6861	11	35	38	8	10	12	14	13
6D	25008120	9	6861	13	11	9	7	6	6	6	6
6D	25008120	17	6861	5	4	3	3	2	2	2	2
6D	2500612025		6861	1	2	2	2	2	4	-39235	-1111
6D	25008120	1	7861	5	3	1	0	1	1	1	1
6D	25008120	9	7861	1	1	1	1	2	1	1	1
6D	25008120	17	7861	1	1	1	1	1	1	1	1
6D	2500712025		7861	1	1	1	1	1	1	1	-1111
6D	25008120	1	8861	1	1	1	1	1	1	1	1
6D	25008120	9	8861	1	1	0	0	0	0	0	0
6D	25008120	17	8861	0	0	0	0	0	0	0	0
6D	2500712025		8861	0	0	0	0	0	0	0	-1111
6D	50008120	1	3861	93	197	291	330	315	239	162	186
6D	50008120	9	3861	183	183	217	258	278	301	320	357
6D	50008120	17	3861	331	297	267	275	312	347	356	298
6D	5000712025		3861	287	246	255	293	303	300	297	-1111
6D	50008120	1	4861	249	224	234	224	176	123	108	170
6D	50008120	9	4861	105	83	78	285	238	162	147	150
6D	50008120	17	4861	157	182	196	171	151	137	136	129
6D	5000612025		4861	121	113	112	113	110	111	-39235	-1111
6D	50008120	1	5861	107	109	260	312	357	604	717	664
6D	50008120	9	5861	696	911	1098	1028	1017	1070	1105	1116
6D	50008120	17	5861	1095	1010	957	883	770	664	632	614
6D	5000712025		5861	547	491	452	410	396	388	360	-1111
6D	50008120	1	6861	329	306	278	158	135	117	87	114
6D	50008120	9	6861	140	141	145	146	136	128	128	128
6D	50008120	17	6861	128	128	165	145	141	138	136	136

6D	780071202510862	144976	144974	144970	144965	144963	144961	144960	
6D	78008120 111862	144957	144955	144952	144944	144944	144943	144941	144939
6D	78008120 911862	144930	144925	144926	144925	144922	144920	144917	144914
6D	780081201711862	144909	144904	144900	144895	144892	144887	144883	144879
6D	780061202511862	144875	144871	144868	144864	144862	144858		
6D	78008120 112862	144856	144852	144850	144846	144841	144838	144836	144834
6D	78008120 912862	144833	144831	144829	144826	144825	144823	144822	144821
6D	780081201712862	144820	144819	144818	144818	144818	144818	144818	144817
6D	780071202512862	144817	144817	144817	144817	144817	144816	144816	

6S	1111	1200101861	012031011	0
6S	1111	1200102861	012028021	0
6S	1111	1200103861	012031031	0
6S	1111	1200104861	1012030041	10
6S	1111	1200105861	10612031051	106
6S	1111	1200106861	22412030061	224
6S	1111	1200107861	12712031071	127
6S	1111	1200108861	25412031081	254
6S	1111	1200109861	7012030091	70
6S	1111	1200110861	5912031101	59
6S	1111	1200111861	012031111	0
6S	1111	1200112861	012031121	0
6S	2222	1200101861	012031011	0
6S	2222	1200102861	012028021	0
6S	2222	1200103861	012031031	0
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6S	2222	1200105861	12012031051	120
6S	2222	1200106861	22712030061	227
6S	2222	1200107861	12712031071	127
6S	2222	1200108861	25412031081	254
6S	2222	1200109861	7012030091	70
6S	2222	1200110861	6912031101	69
6S	2222	1200111861	012031111	0
6S	2222	1200112861	012031121	0

6S	7176	1200101861	012031011	0
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6S	7176	1200103861	012031031	0
6S	7176	1200104861	-3812030041	-38
6S	7176	1200105861	5712031051	57
6S	7176	1200106861	12912030061	129
6S	7176	1200107861	2912031071	29
6S	7176	1200108861	20512031081	205
6S	7176	1200109861	5912030091	59
6S	7176	1200110861	5912031101	59
6S	7176	1200111861	012031111	0
6S	7176	1200112861	012031121	0
6S	7668	1200101861	012031011	0
6S	7668	1200102861	012028021	0
6S	7668	1200103861	012031031	0
6S	7668	1200104861	-2012030041	-20
6S	7668	1200105861	10412031051	104
6S	7668	1200106861	12112030061	121
6S	7668	1200107861	4712031071	47
6S	7668	1200108861	18312031081	183
6S	7668	1200109861	6712030091	67
6S	7668	1200110861	4412031101	44
6S	7668	1200111861	012031111	0
6S	7668	1200112861	012031121	0


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C115 97797 145800 87000 145900 90000
C116 97797 9999999999999999999999999999999
CC01 97798 0 112 ROUND FALL EVAPORATION
CC02 97798 7668 97797 1081000
CC01 7800 0 ROUND LAKE NEAR WHITEWOOD - RECORDED
C110 7800 10000 10 145200 100
C111 7800 145900 1000 999999999999999
CC01 97805 0 ROUND LAKE NEAR WHITEWOOD - SUMMED
CRO1 97843 1 QUAPPELLE RIVER BELOW ROUND - ROUTED
CRO2 97843 10 331 3100
CC01 97899 0 QUAPPELLE AT TANTALLON SUMMATION POINT
CRO1 97983 1 QUAPPELLE -ROUTED
CRO2 97983 7 329 6110
CC01 97999 1 QUAPPELLE RIVER NEAR WELBY-NATURAL FLOW
CLO1 S97999 1 CUM Q.R. NEAR WELBY
CLO2 S97999 10000 100
C101 S97999 100 -1 -100000 10000 1 100000000
C102 S97999 9999999999999999999999999999999
CC01 8000 0 QUAPPELLE RIVER NEAR WELBY - RECORDED
N Q3
P 01 111
P 02 222
P 03 2776
P 04 7668
P 05 1783
P 05 1782
P 06 1149
P 07 1791
P 08 95549
P 09 97197
P 10 97397
P 11 97697
P 12 97797
P 13 G95804 95804
P 14 G97199 297199
P 15 G97399 297399
P 16 1120 1150
P 17 1150 91350
P 18 91350 1795
P 19 9271 1795
P 20 1795 92800
P 21 92800 92300
P 22 92300 94000
P 23 4000 REMOVED
P 24 94000 94263
P 25 94263 95305
P 26 95305 94900
P 26 5000 REMOVED
P 27 94900 95323
P 28 95323 95335
P 29 95335 95365
P 30 95548 95804
P 31 95804 95365
P 32 5342 REMOVED
P 33 95365 95999
P 34 6000 REMOVED
P 35 86000 REMOVED
P 36 95999 96033
P 37 96033 96965
P 38 96965 96993
P 39 96993 97000
P 40 7000 REMOVED
P 41 97000 97173
P 42 97173 297199
P 43 97198 297199
P 44 297199 97205
P 45 7200 REMOVED
P 46 97205 97213
P 47 97213 97245
P 48 97245 297399
P 49 97398 297399
P 50 297399 97500
P 51 7350 REMOVED
P 52 7500 REMOVED
P 53 97500 97535
P 54 97535 97563
P 55 97563 97565
P 56 97565 97593
P 57 97593 97599
P 58 7600 REMOVED
P 59 97599 97605
P 60 97605 97643
P 61 97643 97665
P 62 97665 297699
P 63 97698 297699
P 64 297699 97755
P 65 7700 REMOVED
P 66 97755 97783
P 67 97783 97795
P 68 97795 297799
P 69 97798 297799
P 70 297799 97805
P 71 7800 REMOVED
P 72 97805 97843
P 72 97843 97899
P 73 97899 97983
P 74 97983 97999
P 75 97999 S97999
P 76 S97999
P 77 8000 REMOVED
T 120010886240120010387000 5112
ZL S97999 120010886 0 1
INITIAL CONDITIONS CARDS (NOT REQUIRED IF RAN AFTER Q12)
2L 95804 12031 786 50 160549
2L 95804 120 1 886 50 160549
2L 1795 12031 775 166599
2L 1795 120 1 875 166595
2L 197199 12031 775 157183

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6S	7668	1200108861	18312031081	183
6S	7668	1200109861	6712030091	67
6S	7668	1200110861	4412031101	44
6S	7668	1200111861	012031111	0
6S	7668	1200112861	012031121	0

APPENDIX E

MODIFIED QU'APPELLE RIVER SSARR NATURAL FLOW MODEL 1986 OUTPUT RESULTS

Spring and Summer Model: Page E-1

Fall and Winter Model: Page E-1

1

STREAMFLOW ROUTING

RUN DATE RUN NO. INITIAL DATE, HOUR
1 MAR 86 1200

SSARR MODEL

			00008000	00007984	00097999	00007999
			QUAPPELLE RIVER	QU'APPELLE R. NE	QUAPPELLE RIVER	QUAPPELLE RIVER
DATE-HOUR			FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS
1	MAR 86	1200	149.	143.	69.	5.
2	MAR 86	1200	152.	143.	72.	8.
3	MAR 86	1200	157.	144.	168.	12.
4	MAR 86	1200	157.	162.	159.	-5.
5	MAR 86	1200	155.	237.	129.	-82.
6	MAR 86	1200	154.	348.	120.	-194.
7	MAR 86	1200	160.	421.	145.	-261.
8	MAR 86	1200	174.	445.	168.	-271.
9	MAR 86	1200	179.	451.	172.	-272.
10	MAR 86	1200	177.	450.	171.	-273.
11	MAR 86	1200	176.	448.	177.	-273.
12	MAR 86	1200	172.	447.	185.	-275.
13	MAR 86	1200	173.	445.	200.	-272.
14	MAR 86	1200	178.	445.	218.	-267.
15	MAR 86	1200	185.	444.	234.	-259.
16	MAR 86	1200	238.	444.	291.	-206.
17	MAR 86	1200	456.	446.	509.	9.
18	MAR 86	1200	600.	455.	652.	145.
19	MAR 86	1200	699.	454.	749.	245.
20	MAR 86	1200	812.	455.	861.	357.
21	MAR 86	1200	826.	466.	873.	360.
22	MAR 86	1200	844.	585.	890.	259.
23	MAR 86	1200	911.	576.	954.	335.
24	MAR 86	1200	1,040.	684.	1,090.	354.
25	MAR 86	1200	1,010.	777.	989.	237.
26	MAR 86	1200	964.	797.	949.	167.
27	MAR 86	1200	985.	799.	974.	186.
28	MAR 86	1200	982.	798.	979.	184.
29	MAR 86	1200	1,040.	794.	1,050.	244.
30	MAR 86	1200	1,150.	788.	1,170.	363.
31	MAR 86	1200	1,390.	793.	1,420.	593.
1	APR 86	1200	1,100.	844.	1,140.	254.
2	APR 86	1200	798.	861.	845.	-63.
3	APR 86	1200	657.	833.	694.	-176.
4	APR 86	1200	569.	758.	597.	-189.
5	APR 86	1200	463.	664.	504.	-201.
6	APR 86	1200	327.	553.	416.	-226.
7	APR 86	1200	268.	438.	426.	-170.
8	APR 86	1200	233.	340.	458.	-107.
9	APR 86	1200	210.	267.	486.	-57.
10	APR 86	1200	187.	219.	496.	-31.
11	APR 86	1200	173.	183.	501.	-10.
12	APR 86	1200	162.	156.	502.	5.
13	APR 86	1200	152.	134.	497.	17.
14	APR 86	1200	143.	119.	487.	24.
15	APR 86	1200	132.	106.	472.	25.
16	APR 86	1200	124.	94.	460.	29.
17	APR 86	1200	122.	86.	453.	35.
18	APR 86	1200	118.	79.	443.	38.

1

STREAMFLOW ROUTING

RUN DATE RUN NO. INITIAL DATE, HOUR
1 MAR 86 1200

SSARR MODEL

			00008000	00007984	00097999	00007999
			QUAPPELLE RIVER	QU'APPELLE R. NE	QUAPPELLE RIVER	QUAPPELLE RIVER
DATE-HOUR			FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS
19	APR 86	1200	113.	77.	430.	35.
20	APR 86	1200	110.	74.	418.	35.
21	APR 86	1200	108.	71.	407.	36.
22	APR 86	1200	104.	67.	392.	36.
23	APR 86	1200	102.	67.	377.	34.
24	APR 86	1200	100.	72.	363.	27.
25	APR 86	1200	97.	67.	353.	30.
26	APR 86	1200	95.	60.	349.	34.
27	APR 86	1200	94.	62.	342.	31.
28	APR 86	1200	91.	66.	327.	24.
29	APR 86	1200	93.	68.	318.	24.
30	APR 86	1200	97.	68.	313.	28.
1	MAY 86	1200	101.	72.	309.	29.
2	MAY 86	1200	98.	76.	296.	21.
3	MAY 86	1200	99.	77.	286.	21.
4	MAY 86	1200	102.	83.	364.	18.
5	MAY 86	1200	112.	104.	386.	7.
6	MAY 86	1200	156.	172.	474.	-16.
7	MAY 86	1200	250.	268.	582.	-17.
8	MAY 86	1200	338.	373.	630.	-35.
9	MAY 86	1200	340.	431.	595.	-91.
10	MAY 86	1200	347.	441.	586.	-93.
11	MAY 86	1200	351.	441.	585.	-90.
12	MAY 86	1200	353.	430.	498.	-77.
13	MAY 86	1200	343.	390.	468.	-46.
14	MAY 86	1200	360.	300.	462.	59.
15	MAY 86	1200	351.	223.	445.	128.
16	MAY 86	1200	345.	171.	446.	174.
17	MAY 86	1200	346.	146.	458.	200.
18	MAY 86	1200	353.	129.	477.	224.
19	MAY 86	1200	341.	120.	475.	221.
20	MAY 86	1200	333.	116.	476.	217.
21	MAY 86	1200	326.	110.	478.	216.
22	MAY 86	1200	314.	103.	477.	211.

23	MAY	86	1200	291.	96.	466.	195.
24	MAY	86	1200	264.	93.	447.	170.
25	MAY	86	1200	260.	92.	449.	168.
26	MAY	86	1200	274.	88.	470.	185.
27	MAY	86	1200	262.	86.	464.	176.
28	MAY	86	1200	242.	86.	449.	156.
29	MAY	86	1200	216.	87.	425.	129.
30	MAY	86	1200	200.	87.	410.	113.
31	MAY	86	1200	181.	87.	392.	93.
1	JUN	86	1200	164.	88.	375.	75.
2	JUN	86	1200	144.	87.	355.	56.
3	JUN	86	1200	132.	88.	343.	43.
4	JUN	86	1200	124.	85.	335.	38.
5	JUN	86	1200	120.	82.	332.	37.
6	JUN	86	1200	114.	80.	328.	34.

1
 RUN DATE RUN NO. INITIAL DATE, HOUR SSARR MODEL
 1 MAR 86 1200

DATE-HOUR	00008000 QUAPPELLE RIVER		00007984 QU'APPELLE R. NE QUAPPELLE RIVER		00097999 QUAPPELLE RIVER	
	FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS
7 JUN 86 1200	107.	76.	320.	30.		
8 JUN 86 1200	93.	73.	305.	19.		
9 JUN 86 1200	81.	74.	289.	6.		
10 JUN 86 1200	79.	75.	282.	3.		
11 JUN 86 1200	73.	73.	270.	-1.		
12 JUN 86 1200	68.	70.	257.	-2.		
13 JUN 86 1200	64.	69.	243.	-5.		
14 JUN 86 1200	62.	69.	228.	-7.		
15 JUN 86 1200	64.	66.	218.	-2.		
16 JUN 86 1200	63.	63.	208.	-1.		
17 JUN 86 1200	60.	61.	198.	-1.		
18 JUN 86 1200	58.	57.	191.	1.		
19 JUN 86 1200	58.	56.	183.	0.		
20 JUN 86 1200	62.	58.	182.	-5.		
21 JUN 86 1200	62.	57.	175.	4.		
22 JUN 86 1200	63.	58.	169.	4.		
23 JUN 86 1200	61.	57.	161.	3.		
24 JUN 86 1200	58.	56.	153.	1.		
25 JUN 86 1200	55.	56.	145.	-1.		
26 JUN 86 1200	56.	58.	140.	-1.		
27 JUN 86 1200	54.	58.	133.	-4.		
28 JUN 86 1200	49.	60.	125.	-11.		
29 JUN 86 1200	48.	59.	121.	-7.		
30 JUN 86 1200	47.	59.	118.	-8.		
1 JUL 86 1200	47.	59.	116.	-8.		
2 JUL 86 1200	45.	59.	110.	-10.		
3 JUL 86 1200	44.	59.	105.	-10.		
4 JUL 86 1200	47.	59.	105.	-5.		
5 JUL 86 1200	54.	59.	111.	-0.		
6 JUL 86 1200	60.	61.	116.	-1.		
7 JUL 86 1200	61.	61.	118.	0.		
8 JUL 86 1200	70.	65.	129.	13.		
9 JUL 86 1200	67.	62.	129.	9.		
10 JUL 86 1200	62.	62.	122.	-1.		
11 JUL 86 1200	60.	66.	116.	-6.		
12 JUL 86 1200	60.	67.	110.	-7.		
13 JUL 86 1200	60.	61.	108.	-1.		
14 JUL 86 1200	75.	63.	123.	12.		
15 JUL 86 1200	75.	64.	126.	10.		
16 JUL 86 1200	85.	63.	138.	21.		
17 JUL 86 1200	84.	65.	139.	18.		
18 JUL 86 1200	73.	68.	129.	4.		
19 JUL 86 1200	69.	71.	123.	-2.		
20 JUL 86 1200	78.	72.	129.	5.		
21 JUL 86 1200	88.	71.	136.	17.		
22 JUL 86 1200	90.	67.	136.	22.		
23 JUL 86 1200	86.	61.	131.	24.		
24 JUL 86 1200	83.	58.	126.	24.		
25 JUL 86 1200	81.	58.	122.	22.		

1
 RUN DATE RUN NO. INITIAL DATE, HOUR SSARR MODEL
 1 MAR 86 1200

DATE-HOUR	00008000 QUAPPELLE RIVER		00007984 QU'APPELLE R. NE QUAPPELLE RIVER		00097999 QUAPPELLE RIVER	
	FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS	FLOW CFS
26 JUL 86 1200	92.	58.	130.	33.		
27 JUL 86 1200	99.	60.	134.	38.		
28 JUL 86 1200	108.	59.	141.	49.		
29 JUL 86 1200	118.	59.	151.	58.		
30 JUL 86 1200	104.	58.	138.	45.		
31 JUL 86 1200	104.	57.	139.	46.		
1 AUG 86 1200	97.	58.	133.	38.		

STREAMFLOW ROUTING

RUN DATE RUN NO. INITIAL DATE, HOUR
1 AUG 86 1200

DATE-HOUR	00001150	00001795	00095804	00297199	00297399	00297699	00297799	00097999	S97999
	EYEBROW LAKE NAT ELEVATION FEET-MSL	BUFFALO POUND ELEVATION FEET-MSL	LAST MOUNTAIN ELEVATION FEET-MSL	ECHO LAKE AT ELEVATION FEET-MSL	FIS ELEVATION FEET-MSL	KATEPWA LAKE AT ELEVATION FEET-MSL	CROOKED LAKE NEA ELEVATION FEET-MSL	ROUND LAKE NEAR ELEVATION FEET-MSL	QUAPPELLE RIVER FLOW CFS
13 FEB 87 1200	1.719.30	1.661.17	1.604.59	1.568.46	1.568.44	1.479.61	1.448.12	-1.	5.476.
14 FEB 87 1200	1.719.30	1.661.17	1.604.59	1.568.46	1.568.44	1.479.61	1.448.12	-1.	5.476.
15 FEB 87 1200	1.719.30	1.661.17	1.604.59	1.568.47	1.568.45	1.479.61	1.448.12	-1.	5.476.
16 FEB 87 1200	1.719.30	1.661.17	1.604.59	1.568.47	1.568.45	1.479.62	1.448.12	-1.	5.476.
17 FEB 87 1200	1.719.30	1.661.17	1.604.59	1.568.47	1.568.46	1.479.62	1.448.12	-1.	5.476.
18 FEB 87 1200	1.719.30	1.661.17	1.604.59	1.568.48	1.568.46	1.479.62	1.448.12	-1.	5.476.
19 FEB 87 1200	1.719.30	1.661.17	1.604.59	1.568.48	1.568.46	1.479.62	1.448.12	-1.	5.476.
20 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.49	1.568.47	1.479.62	1.448.12	-1.	5.476.
21 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.49	1.568.47	1.479.62	1.448.12	-1.	5.476.
22 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.49	1.568.48	1.479.62	1.448.12	-1.	5.476.
23 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.50	1.568.48	1.479.62	1.448.12	-1.	5.476.
24 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.50	1.568.48	1.479.62	1.448.12	-1.	5.476.
25 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.51	1.568.49	1.479.62	1.448.12	-1.	5.476.
26 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.51	1.568.49	1.479.62	1.448.12	-1.	5.476.
27 FEB 87 1200	1.719.30	1.661.17	1.604.60	1.568.51	1.568.50	1.479.62	1.448.12	-1.	5.476.
28 FEB 87 1200	1.719.30	1.661.17	1.604.61	1.568.52	1.568.50	1.479.62	1.448.12	-1.	5.476.
1 MAR 87 1200	1.719.30	1.661.17	1.604.61	1.568.52	1.568.50	1.479.62	1.448.12	-1.	5.476.